

DEPARTMENT OF TRANSPORTATION 1401 EAST BROAD STREET RICHMOND, VIRGINIA 23219-2000

PHILIP A. SHUCET
COMMISSIONER

MOHAMMAD MIRSHAHI, P.E.
STATE LOCATION AND DESIGN ENGINEER

October 11, 2002

Memorandum

To: All Holders of the Virginia Department of Transportation's 2001 Road and Bridge Standards

Attached is a copy of the new TC-5.01 Superelevation Standards, SD-4 and SD-5 Sight Distance Standards that are to be added to your copy the standards. These new standards were developed to conform with Chapter 3, "Elements of Design" of AASHTO's Fourth Edition of <u>A Policy on Geometric Design of Highways and Streets</u> ("Green Book"). These new standards are to be used in place of the TC-5, Sd2 and SD-3 Standards on all projects beginning with the March 2003 Advertisement. On projects where the design has progressed to a point where it would be impracticable to apply the new standards, such as requiring a redesign of a bridge or acquisition of additional right of way, please contact the State Location and Design Engineer for a design exception. Where feasible, the roadway design may be changed to incorporate the new standards for all projects with Advertisement Dates prior to March 2003.

The new TC-5.01 Superelevation Standard has incorporated the following changes to the new Green Book:

- Two percent (2%) normal pavement crown rate.
- The "Two Second Rule" is applied only for rural conditions with pavement widening.
- Uses the AASHTO method to compute Crown Run-off values.
- Deleted sixteen foot (16') pavement width.
- Added sixteen (16') and eighteen (18') foot pavement widths for interchange ramps.

These changes will affect the length of transition (LS) need from normal pavement crown to required superelevation.

The new SD-4 and SD-5 Sight Distance Standards incorporate the following changes to the new Green Book:

- Increased height of object for the stopping sight distances (SD-4).
- Reduced the height of object for the passing sight distances (SD-5).

These changes will affect the stopping and passing sight distances for vertical curves.

Please add the new TC-5.01 Superelevation Standards, SD-4 and SD-5 Sight Distance Standards to your copy of the 2001 <u>Road and Bridge Standards</u>. If you have any questions or comments regarding this revision to the 2001 <u>Road and Bridge Standards</u>, please contact Nancy Berry of the Engineering Services Section at (804) 786-2543.

Sincerely,

Mohammad Mirshahi, P.E. State Location and Design Engineer

des des		Wr	nen S>L;	S = 107	9.15 +	<u>L</u> 2			Whe	n S <l; s<="" th=""><th>= 46.45</th><th>4 √ <u>L</u></th><th></th><th>S = 9</th><th>ight Dist</th><th>ance in</th><th>Feet</th><th></th><th>Sheet</th><th>1 of 2</th><th>aic nce des sent</th></l;>	= 46.45	4 √ <u>L</u>		S = 9	ight Dist	ance in	Feet		Sheet	1 of 2	aic nce des sent
- Algebraic Difference of Grades in Percent							L	- Length	of Ver	tical Curv	ve in Fe	et									Agebraic Difference of Grades in Percent
4	50	100	150	200	250	300	350	400	450	500	550	600	650	700	750	800	850	900	950	1000	4 4
2.0	565	590	615	640	665	690	715	740	765	790	815	840	865	890	915	940	965	990	1015	1040	2.0
2.5	457	482	507	532	557	582	607	632	657	682	707	732	757	782	807	832	857	881	906	929	2.5
3.0	385	410	435	460	485	510	535	560	585	610	635	660	685	710	735	759	782	805	827	848	3.0
3.5	333	358	383	408	433	458	483	508	533	558	583	608	633	657	680	702	724	745	765	785	3.5
4.0	295	320	345	370	395	420	445	470	495	520	545	569	592	615	636	657	677	697	716	735	4.0
4.5	265	290	315	340	365	390	415	440	465	490	514	536	558	579	600	619	638	657	675	692	4.5
5.0	241	266	291	316	341	366	391	416	441	465	487	509	530	550	569	588	606	623	640	657	5.0
5.5	221	246	271	296	321	346	371	396	420	443	465	485	505	524	542	560	577	594	611	626	5.5
6.0	205	230	255	280	305	330	355	379	402	424	445	465	484	502	519	536	553	569	585	600	6.0
6.5	191	216	241	266	291	316	341	364	387	407	427	446	465	482	499	515	531	547	562	576	6.5
7.0	179	204	229	254	279	304	328	351	372	393	412	430	448	465	481	497	512	527	541	555	7.0
7.5	169	194	219	244	269	294	317	339	360	379	398	415	432	449	465	480	495	509	523	536	7.5
8.0	160	185	210	235	260	284	307	328	348	367	385	402	419	435	450	465	479	493	506	519	8.0
8.5	152	177	202	227	252	276	298	319	338	356	374	390	406	422	436	451	465	478	491	504	8.5
9.0	145	170	195	220	245	268	290	310	328	346	363	379	395	410	424	438	451	465	477	490	9.0
9.5	139	164	189	214	238	261	282	301	320	337	353	369	384	399	413	426	439	452	465	477	9.5
10.0	133	158	183	208	232	254	275	294	312	328	345	360	375	389	402	415	428	441	453	465	10.0
10.5	128	153	178	203	227	248	268	287	304	321	336	351	365	379	393	405	418	430	442	453	10.5
11.0	123	148	173	198	221	243	262	280	297	313	328	343	357	371	384	396	408	420	432	443	11.0
11.5	119	14 4	169	194	217	237	256	274	291	306	321	336	349	362	375	387	399	411	422	433	11.5
12.0	115	140	165	190	212	232	251	268	284	300	314	328	342	355	367	379	391	402	413	424	12.0
12.5	111	136	161	186	208	228	246	263	279	294	308	322	335	348	360	372	383	394	405	415	12.5
13.0	108	133	158	182	204	223	241	258	273	288	302	316	328	342	353	364	376	387	397	407	13.0
13.5	105	130	155	179	200	219	237	253	268	283	297	310	322	335	346	358	369	379	390	400	13.5
14.0	102	127	152	176	196	215	232	248	263	278	291	304	317	328	340	351	362	372	383	393	14.0
14.5	99	124	149	173	193	211	228	244	259	273	286	299	311	323	334	345	356	366	376	386	14.5
15.0	97	122	147	170	190	208	224	240	254	268	281	294	306	317	328	339	350	360	370	379	15.0
16.0	92	117	142	164	184	201	217	232	246	260	272	284	296	307	318	328	339	348	358	367	16.0
17.0	88	113	138	159	178	195	211	225	239	252	264	276	287	298	309	319	328	338	347	356	17.0
18.0	85	110	134	155	173	190	205	219	232	245	257	268	279	290	300	310	319	328	337	346	18.0
19.0	82	107	131	151	169	185	199	213	226	238	250	261	272	282	292	301	311	320	328	337	19.0
20.0	79	104	127	147	164	180	194	208	220	232	244	254	265	275	284	294	303	312	320	328	19.0

NEW 10/02

HEIGHT OF EYE = 3.5 FEET

HEIGHT OF OBJECT = 2.00 FEET

Algebraic Differance of Grades in Percent		Whe	en S>L; S	S = 107	7 <u>9.15</u> +	<u>L</u> 2			When S	(L; S = 4	6.454√	L		S = Sigh	ıt Distano	ce in Fee	et		Sheet 2	of 2	ance dades cent
П							L =	Length o	f Vertica	l Curve	in Feet										Algebraic Differance of Grades in Percent
⋖	1050	1100	1150	1200	1250	1300	1350	1400	1450	1500	1550	1600	1650	1700	1750	1800	1850	1900	1950	2000	∢
2.0	1065	1089	1114	1138	1161	1184	1207	1229	1251	1272	1293	1314	1334	1354	1374	1394	1413	1432	1451	1469	2.0
2.5	952	974	996	1018	1039	1059	1079	1099	1119	1138	1157	1175	1193	1211	1229	1246	1264	1281	1297	1314	2.5
3.0	869	890	910	929	948	967	985	1004	1021	1039	1056	1073	1089	1106	1122	1138	1154	1169	1184	1199	3.0
3.5	805	824	842	860	878	895	912	929	946	962	978	993	1009	1024	1039	1053	1068	1082	1096	1110	3.5
4.0	753	770	788	805	821	837	853	869	884	900	914	929	943	958	972	985	999	1012	1026	1039	4.0
4.5	710	726	743	759	774	790	805	819	834	848	862	876	890	903	916	929	942	955	997	979	4.5
5.0	673	689	705	720	735	749	763	777	791	805	818	831	844	857	869	881	894	906	917	929	5.0
5.5	642	657	672	686	700	714	728	741	754	767	780	792	805	817	829	840	852	863	875	886	5.5
6.0	615	629	643	657	671	684	697	710	722	735	747	759	770	782	793	805	816	827	837	848	6.0
6.5	590	604	618	631	644	657	669	682	694	706	717	729	740	751	762	773	784	794	805	815	6.5
7.0	569	582	595	608	621	633	645	657	669	680	691	702	713	724	735	745	755	765	775	785	7.0
7.5	550	563	575	588	600	612	623	635	646	657	668	679	689	699	710	720	730	739	749	759	7.5
8.0	532	545	557	569	581	592	603	615	625	636	647	657	667	677	687	697	706	716	725	735	8.0
8.5	516	528	540	552	563	574	585	596	607	617	627	637	647	657	667	676	685	695	704	713	8.5
9.0	502	514	525	536	547	558	569	579	590	600	610	619	629	638	648	657	666	675	684	692	9.0
9.5	488	500	511	522	533	543	554	564	574	584	593	603	612	621	630	639	648	657	666	674	9.5
10.0	476	487	498	509	519	530	540	550	559	569	578	588	597	606	615	623	632	640	649	657	10.0
10.5	465	475	486	497	507	517	527	536	546	555	564	573	582	591	600	608	617	625	633	641	10.5
11.0	454	465	475	485	495	505	515	524	533	542	551	560	569	577	586	594	602	611	619	626	11.0
11.5	444	454	465	475	484	494	503	513	522	531	539	548	556	565	573	581	589	597	605	613	11.5
12.0	435	445	455	465	474	484	493	502	511	519	528	536	545	553	561	569	577	585	592	600	12.0
12.5	426	436	446	455	465	474	483	492	500	509	517	526	534	542	550	557	565	573	580	588	12.5
13.0	417	427	437	446	456	465	473	482	491	499	507	515	523	531	539	547	554	562	569	576	13.0
13.5	410	419	429	438	447	456	465	473	481	490	498	506	514	521	529	536	544	551	558	565	13.5
14.0	402	412	421	430	439	448	456	465	473	481	489	497	504	512	519	527	534	541	548	555	14.0
14.5	395	405	414	423	431	440	448	456	465	472	480	488	496	503	510	518	525	532	539	546	14.5
15.0	389	398	407	415	424	432	441	449	457	465	472	480	487	495	502	509	516	523	530	536	15.0
16.0	376	385	394	402	411	419	427	435	442	450	457	465	472	479	486	493	500	506	513	519	16.0
17.0	365	374	382	390	398	406	414	422	429	436	444	451	458	465	471	478	485	491	498	504	17.0
18.0	355	363	371	379	387	395	402	410	417	424	431	438	445	451	458	465	471	477	484	490	18.0
19.0	345	353	361	369	377	384	392	399	406	413	420	426	433	439	446	452	458	465	471	477	19.0
20.0	337	345	352	360	367	375	382	389	396	402	409	415	422	428	435	441	447	453	459	465	20.0

HEIGHT OF EYE = 3.5 FEET

HEIGHT OF OBJECT = 2.00 FEET

NEW 10/02

ence ades cent		Whe	en S>L;S	S = 14	00 +	<u>L</u>			Wher	n SKL; S	= 52.915	$5\sqrt{\frac{L}{A}}$		S = S	iight Dist	ance in I	Feet		Sheet	1 of 2	aic ence ades cent
= Algebraic Difference of Grades in Percent							L	= Length	of Ver	tical Curv	ve in Fee	et									Algebraic Difference of Grades in Percent
Å	50	100	150	200	250	300	350	400	450	500	550	600	650	700	750	800	850	900	950	1000	∀
2.0	725	750	775	800	825	850	875	900	925	950	975	1000	1025	1050	1075	1100	1125	1150	1175	1200	2.0
2.5	585	610	635	660	685	710	735	760	785	810	835	860	885	910	935	960	985	1010	1035	1060	2.5
3.0	492	517	542	567	592	617	642	667	692	717	742	767	792	817	842	867	892	917	942	966	3.0
3.5	425	450	475	500	525	550	575	600	625	650	675	700	725	750	775	800	825	849	872	894	3.5
4.0	375	400	425	450	475	500	525	550	575	600	625	650	675	700	725	748	771	794	815	837	4.0
4.5	336	361	386	411	436	461	486	511	536	561	586	611	636	660	683	706	727	748	769	789	4.5
5.0	305	330	355	380	405	430	455	480	505	530	555	580	603	626	648	669	690	710	729	748	5.0
5.5	280	305	330	355	380	405	430	455	480	505	529	553	575	597	618	638	658	677	695	714	5.5
6.0	258	283	308	333	358	383	408	433	458	483	507	529	551	572	592	611	630	648	666	683	6.0
6.5	240	265	290	315	340	365	390	415	440	464	487	508	529	549	568	587	605	623	640	656	6.5
7.0	225	250	275	300	325	350	375	400	424	447	469	490	510	529	548	566	583	600	616	632	7.0
7.5	212	237	262	287	312	337	362	386	410	432	453	473	493	511	529	547	563	580	596	611	7.5
8.0	200	225	250	275	300	325	350	374	397	418	439	458	477	495	512	529	545	561	577	592	8.0
8.5	190	215	240	265	290	315	340	363	385	406	426	445	463	480	497	513	529	544	559	574	8.5
9.0	181	206	231	256	281	306	330	353	374	394	414	432	450	467	483	499	514	529	544	558	9.0
9.5	172	197	222	247	272	297	321	343	364	384	403	421	438	454	470	486	501	515	529	543	9.5
10.0	165	190	215	240	265	290	313	335	355	374	392	410	427	443	458	473	488	502	516	529	10.0
10.5	158	183	208	233	258	283	306	327	346	365	383	400	416	432	447	462	476	490	503	516	10.5
11.0	152	177	202	227	252	276	298	319	338	357	374	391	407	422	437	451	465	479	492	505	11.0
11.5	147	172	197	222	247	270	292	312	331	349	366	382	398	413	427	441	455	468	481	493	11.5
12.0	142	167	192	217	242	265	286	306	324	342	358	374	389	404	418	432	445	458	471	483	12.0
12.5	137	162	187	212	237	259	280	299	317	335	351	367	382	396	410	423	436	449	461	473	12.5
13.0	133	158	183	208	232	254	275	294	311	328	344	359	374	388	402	415	428	440	452	464	13.0
13.5	129	154	179	204	228	249	269	288	306	322	338	353	367	381	394	407	420	432	444	455	13.5
14.0	125	150	175	200	224	245	265	283	300	316	332	346	361	374	387	400	412	424	436	447	14.0
14.5	122	147	172	197	220	241	260	278	295	311	326	340	354	368	381	393	405	417	428	439	14.5
15.0	118	143	168	193	216	237	256	273	290	306	320	335	348	361	374	386	398	410	421	432	15.0
16.0	113	138	163	187	209	229	247	265	281	296	310	324	337	350	362	374	386	397	408	418	16.0
17.0	107	132	157	181	203	222	240	257	272	287	301	314	327	340	351	363	374	385	396	406	17.0
18.0	103	128	153	176	197	216	233	249	265	279	292	306	318	330	342	353	364	374	384	394	18.0
19.0	99	124	149	172	192	210	227	243	258	271	285	297	309	321	332	343	354	364	374	384	19.0
20.0	95	120	145	167	187	205	221	237	251	265	277	290	302	313	324	335	345	355	365	374	20.0

NEW 10/02

608.08

HEIGHT OF EYE = 3.5 FEET

HEIGHT OF OBJECT = 3.5 FEET

Algebraic Differance of Grades in Percent		Whe	en S>L;S	= 140	00 + <u>L</u>				When S	(L; S = 5	2.915 √	L A		S = Sigh	nt Distanc	ce in Fee	;t		Sheet 2	of 2	Algebraic Differance O of Grades G in Percent
П							L = I	_ength o	f Vertica	l Curve	in Feet										Algeb Differ of Gr
⋖	1050	1100	1150	1200	1250	1300	1350	1400	1450	1500	1550	1600	1650	1700	1750	1800	1850	1900	1950	2000	
2.0	1225	1250	1275	1300	1325	1350	1375	1400	1425	1449	1473	1497	1520	1543	1565	1587	1609	1631	1652	1673	2.0
2.5	1085	1100	1135	1159	1183	1207	1230	1252	1274	1296	1318	1339	1359	1380	1400	1420	1439	1459	1478	1497	2.5
3.0	990	1013	1036	1058	1080	1102	1122	1143	1163	1183	1203	1222	1241	1260	1278	1296	1314	1332	1349	1366	3.0
3.5	917	938	959	980	1000	1020	1039	1058	1077	1095	1114	1131	1149	1166	1183	1200	1217	1233	1249	1265	3.5
4.0	857	877	897	917	935	954	972	990	1007	1025	1042	1058	1075	1091	1107	1122	1138	1153	1168	1183	4.0
4.5	808	827	846	864	882	899	917	933	950	966	982	998	1013	1028	1043	1058	1073	1087	1102	1116	4.5
5.0	767	785	802	820	837	853	869	885	901	917	932	947	961	976	990	1004	1018	1032	1045	1058	5.0
5.5	731	748	765	782	798	814	829	844	859	874	888	903	917	930	944	957	970	983	996	1009	5.5
6.0	700	716	733	748	764	779	794	808	823	837	850	864	877	891	904	917	929	942	954	966	6.0
6.5	673	688	704	719	734	748	763	777	790	804	817	830	843	856	868	881	893	905	917	928	6.5
7.0	648	663	678	693	707	721	735	748	762	775	787	800	812	825	837	849	860	872	883	894	7.0
7.5	626	641	655	669	683	697	710	723	736	748	761	773	785	797	808	820	831	842	853	864	7.5
8.0	606	620	634	648	661	675	687	700	712	725	737	748	760	771	783	794	805	815	826	837	8.0
8.5	588	602	615	629	642	654	667	679	691	703	715	726	737	748	759	770	781	791	801	812	8.5
9.0	572	585	598	611	624	636	648	660	672	683	694	706	716	727	738	748	759	769	779	789	9.0
9.5	556	569	582	595	607	619	631	642	654	665	676	687	697	708	718	728	738	748	758	768	9.5
10.0	542	555	567	580	592	603	615	626	637	648	659	669	680	690	700	710	720	729	739	748	10.0
10.5	529	542	554	566	577	589	600	611	622	632	643	653	663	673	683	693	702	712	721	730	10.5
11.0	517	529	541	553	564	575	586	597	608	618	628	638	648	658	667	677	686	695	705	714	11.0
11.5	506	518	529	541	552	563	573	584	594	604	614	624	634	643	653	662	671	680	689	698	11.5
12.0	495	507	518	529	540	551	561	572	582	592	601	611	620	630	639	648	657	666	675	683	12.0
12.5	485	496	508	518	529	540	550	560	570	580	589	599	608	617	626	635	644	652	661	669	12.5
13.0	476	487	498	508	519	529	539	549	559	568	578	587	596	605	614	623	631	640	648	656	13.0
13.5	467	478	488	499	509	519	529	539	548	558	567	576	585	594	602	611	619	628	636	644	13.5
14.0	458	469	480	490	500	510	520	529	539	548	557	566	574	583	592	600	608	616	624	632	14.0
14.5	450	461	471	481	491	501	511	520	529	538	547	556	564	573	581	590	598	606	614	621	14.5
15.0	443	453	463	473	483	493	502	511	520	529	538	547	555	563	572	580	588	596	603	611	15.0
16.0	429	439	449	458	468	477	486	495	504	512	521	529	537	545	553	561	569	577	584	592	16.0
17.0	416	426	435	445	454	463	472	480	489	497	505	513	521	529	537	544	552	559	567	574	17.0
18.0	404	414	423	432	441	450	458	467	475	483	491	499	507	514	522	529	536	544	551	558	18.0
19.0	393	403	412	421	429	438	446	454	462	470	478	486	493	501	508	515	522	529	536	543	19.0
20.0	383	392	401	410	418	427	435	443	451	458	466	473	481	488	495	502	509	516	522	529	20.0

HEIGHT OF EYE = 3.5 FEET HEIGHT OF OBJECT = 3.5 FEET

VIRGINIA DEPARTMENT OF TRANSPORTATION

NEW 10/02

STANDARD SYMBOLS

LOCATION D
P.R.C
DVAPPROXIMATE MAXIMUM SAFE SPEED IN MILES PER HOUR USING STANDARD RATE OF SUPER- ELEVATION.
NCAPPROXIMATE MAXIMUM SAFE SPEED IN MILES PER HOUR WITH NO SUPERELEVATION.
LSLENGTH OF TRANSITION CURVE MEASURED ALONG BASELINE. WHERE NO TRANSITION CURVE IS APPLIED LS IS LENGTH OF SUPERELEVATION TRANSITION.
W OR PWWIDTH OF STANDARD PAVEMENT.
ZTDISTANCE FROM TRANSITIONED BASELINE TO EDGES OF TRANSITIONED PAVEMENT
WMAXIMUM TOTAL PAVEMENT WIDENING. ERATE OF SUPERELEVATION.
ERATE OF SUPERELEVATION. FSAFE SIDE FRICTION FACTOR.
S
CDIFFERENCE IN ELEVATION BETWEEN BASELINE (CENTER) AND EDGE OF PAVEMENT FOR
STANDARD PAVEMENT CROWN.
CRSTANDARD PAVEMENT CROWN TRANSITION OR CROWN RUNOFF LENGTH.
CP
NPCNORMAL PAVEMENT CROWN.

ALL DISTANCES (HORIZONTAL AND VERTICAL) ARE MEASURED IN FEET.

SPECIFICATION REFERENCE TRANSITION CURVES FOR RURAL AND URBAN HIGHWAYS AND STREET CONDITIONS

URBAN CONDITION

URBAN CONDITIONS APPLY TO URBAN <u>STREET</u> SYSTEMS AND ANY OTHER ROAD WITH PRESENT OR FUTURE URBAN STREET OPERATING CONDITIONS.

THESE TABLES CONTAIN THE MINIMUM SUPERELEVATION RATES AND TRANSITION LENGTHS FOR STANDARD URBAN PAVEMENT WIDTHS THROUGH A RANGE OF DESIGN VELOCITIES CONSIDERED MOST LIKELY TO BE USED IN URBAN ROAD DESIGN.

DEFINITIONS FOR THE STANDARD SYMBOLS USED THROUGHOUT THESE TABLES ARE FOUND ON SHEET 802.01.

A TABLE FOR "LOW SPEED URBAN" DESIGNS IS ON SHEET 802.24 WITH A RANGE OF STANDARD PAVEMENT WIDTHS (W), TRANSITION LENGTHS (LS), AND RADII OF CURVE WHEN SUPERELEVATED BY AN AMOUNT EQUAL TO THE NORMAL CROWN AND THE APPROXIMATE MAXIMUM SAFE SPEEDS (DV) AFFORDED THEREBY. VALUES IN THIS TABLE CAN BE USED ON STREETS WITH OPERATING SPEEDS LESS THAN OR EQUAL TO 45 MPH. ALSO SHOWN ARE THE APPROXIMATE MAXIMUM SAFE SPEEDS (NC) WITH NO SUPERELEVATION. VALUES FOR (NC) CAN BE USED ON URBAN ARTERIAL, COLLECTOR, AND LOCAL STREETS.

FOR MINIMUM DESIGN FACTORS FOR VARIOUS DESIGN SPEEDS FOR URBAN CONDITIONS SEE SHEETS 802.25 THRU 802.33

WHEN URBAN CONDITIONS APPLY THERE <u>WILL</u> BE NO BASELINE TRANSITION OR PAVEMENT WIDENING. THE LENGTH OF TRANSITION (LS) DETERMINES THE LENGTH OF SUPERELEVATION TRANSITION THROUGH WHICH THE OUTER EDGE OF PAVEMENT IS RAISED ABOVE THE BASELINE GRADE TO A MAXIMUM OF E ($\frac{W}{2}$). SEE SHEET 802.07 FOR A GRAPHICAL ILLUSTRATION OF THE APPLICATION OF THIS CORRECTION.

FOR CURVE RADII NOT LISTED IN TABLES REFER TO SHEET 802.22 TO CALCULATE TRANSITION LENGTHS (LS).

LS SHOULD BE SHOWN ON THE PLANS FOR ALL CURVES.

E SHOULD BE SHOWN ON THE PLANS FOR ALL CURVES WITH URBAN STREET CONDITIONS.

FOR GRAPHICAL ILLUSTRATION OF DESIGN SUPERELEVATION RATES FOR URBAN CONDITIONS SEE SHEET 802.19.

FOR ADDITIONAL GENERAL INSTRUCTIONS (BOTH URBAN AND RURAL) SEE SHEET 802.04.

EXPLANATION OF TABLES AND INSTRUCTIONS FOR USE URBAN CONDITION

RURAL CONDITION

RURAL CONDITIONS APPLY TO INTERSTATE, ARTERIAL, PRIMARY AND SECONDARY SYSTEMS OR TO ANY OTHER ROAD WITH RURAL TYPE DESIGN AND OPERATING CONDITIONS.

THESE TABLES CONTAIN THE MINIMUM ALLOWABLE SUPERELEVATION, TRANSITION LENGTHS, AND WIDENING CORRECTIONS FOR STANDARD RURAL PAVEMENT WIDTHS THROUGH A RANGE OF DESIGN VELOCITIES CONSIDERED MOST LIKELY TO BE USED IN RURAL HIGHWAY DESIGN.

DEFINITIONS FOR THE STANDARD SYMBOLS USED THROUGHOUT THESE TABLES ARE FOUND ON SHEET 802.01.

FOR MINIMUM DESIGN FACTORS FOR VARIOUS DESIGN SPEEDS FOR RURAL CONDITIONS SEE SHEETS 802.34 THRU 802.44.

ON CURVES WITH GREATER THAN 2865 FT RADIUS, THERE WILL BE NO SPIRAL TRANSITION OR PAVEMENT WIDENING. PAVEMENT WILL BE SUPERELEVATED BY AN AMOUNT EQUAL TO THE RATE SHOWN IN THE TABLES. SEE SHEET 802.06 FOR A GRAPHICAL ILLUSTRATION OF THE APPLICATION OF THIS CORRECTION.

ON CURVES WITH PAVEMENT WIDTHS OF 24'OR WIDER AND A RADIUS OF 882 FT. OR GREATER, THERE WILL BE NO SPIRAL TRANSITION OR PAVEMENT WIDENING. PAVEMENT WILL BE SUPERELEVATED BY AN AMOUNT EQUAL TO THE RATE SHOWN IN THESE TABLES.

FOR CURVE RADII NOT LISTED IN TABLES, REFER TO SHEET 802.22 TO CALCULATE TRANSITION LENGTHS (LS) AND PAVEMENT WIDENING (w).

LS AND E SHOULD BE SHOWN ON THE PLANS FOR ALL CURVES ..

FOR GRAPHICAL ILLUSTRATION OF DESIGN SUPERELEVATION RATES FOR RURAL CONDITIONS SEE SHEET 802.20.

FOR ADDITIONAL GENERAL INSTRUCTIONS (BOTH URBAN AND RURAL) SEE SHEET 802.04.

SEE SHEET 802.05 FOR A GRAPHICAL ILLUSTRATION OF SPIRAL TRANSITIONS.

EXPLANATION OF TABLES AND INSTRUCTIONS FOR USE RURAL CONDITION

GENERAL CONDITION

ALL ORIGINAL CROSS SECTIONS SHALL BE TAKEN FROM THE BASELINE AT STATIONS, PLUS FIFTIES, AND UNUSUAL BREAKS IN THE GROUND AS ON TANGENT ALIGNMENT.

WHERE A PART OR ALL OF A SUPERELEVATION TRANSITION CURVE FALLS ON A VERTICAL CURVE, ELEVATIONS ON THE VERTICAL CURVE SHOULD BE COMPUTED FOR THE POSITIONS GIVEN ON SHEET 802.16 FOR CROWN TRANSITIONS, SHEET 802.17 FOR URBAN PROJECTS AND SHEET 802.18 FOR RURAL PROJECTS. THESE ELEVATIONS AND PLUSES SHOULD BE SHOWN ON THE PLANS FOR THE CONVENIENCE OF THE SURVEY PARTY IN STAKING OUT THE PROJECT. THROUGHOUT THESE SECTIONS OF THE GRADE, ELEVATIONS AT EVEN STATIONS AND PLUS FIFTIES SHOULD BE OMITTED.

SLOPE STAKES SHOULD BE SET AT THE POSITIONS ON THE TRANSITION GIVEN ON SHEETS 802.16, 802.17 AND 802.18 AND GROUND CROSS SECTIONS TAKEN AT THESE POSITIONS OMITTING THE STATIONS AND PLUS FIFTIES THROUGHOUT THE TRANSITION. IF UNUSUAL BREAKS IN THE GROUND OCCUR, ADDITIONAL SECTIONS SHOULD, OF COURSE, BE TAKEN. ADDITIONAL SECTIONS SHOULD ALSO BE TAKEN WHERE LOCATION IS THROUGH ROCK CUT IN ANTICIPATION OF UNUSUAL BREAKAGE WHICH MAY OCCUR DURING CONSTRUCTION.

AFTER ROUGH GRADING HAS BEEN DONE, FINE GRADING (BLUE TOP) AND FORM STAKES SHOULD BE SET AT THE POSITIONS GIVEN ON SHEET 802.16 FOR CROWN TRANSITIONS, SHEET 802.17 FOR URBAN PROJECTS OR AS GIVEN ON SHEET 802.18 FOR RURAL PROJECTS.

FINAL CROSS SECTIONS SHOULD, OF COURSE, BE TAKEN AT THOSE POSITIONS AT WHICH THE SLOPE STAKE SECTIONS WERE TAKEN. WHERE UNUSUAL BREAKAGE IN ROCK OCCURS, AND THIS WAS NOT ANTICIPATED, ADDITIONAL FINAL SECTIONS SHOULD BE TAKEN AND ORIGINAL GROUND SECTIONS INTERPOLATED.

BASELINE STAKES SHOULD BE SET AT ALL P.C.'S, P.T.'S, T.S.'S, S.T.'S, S.C.'S, AND C.S.'S IN STAKING OUT ALIGNMENT BUT SLOPE STAKES NEED NOT BE SET NOR CROSS SECTIONS TAKEN AT P.C.'S OR P.T.'S EXCEPT WHERE CALLED FOR IN THE ACCOMPANYING TABLES. THE TRANSITION WILL TAKE ITS FORM FROM THE POSITIONS GIVEN ON SHEETS 802.17 AND 802.18.

THE RIGHT OF WAY SHALL, IN ALL CASES, BE REFERENCED FROM THE BASELINE.

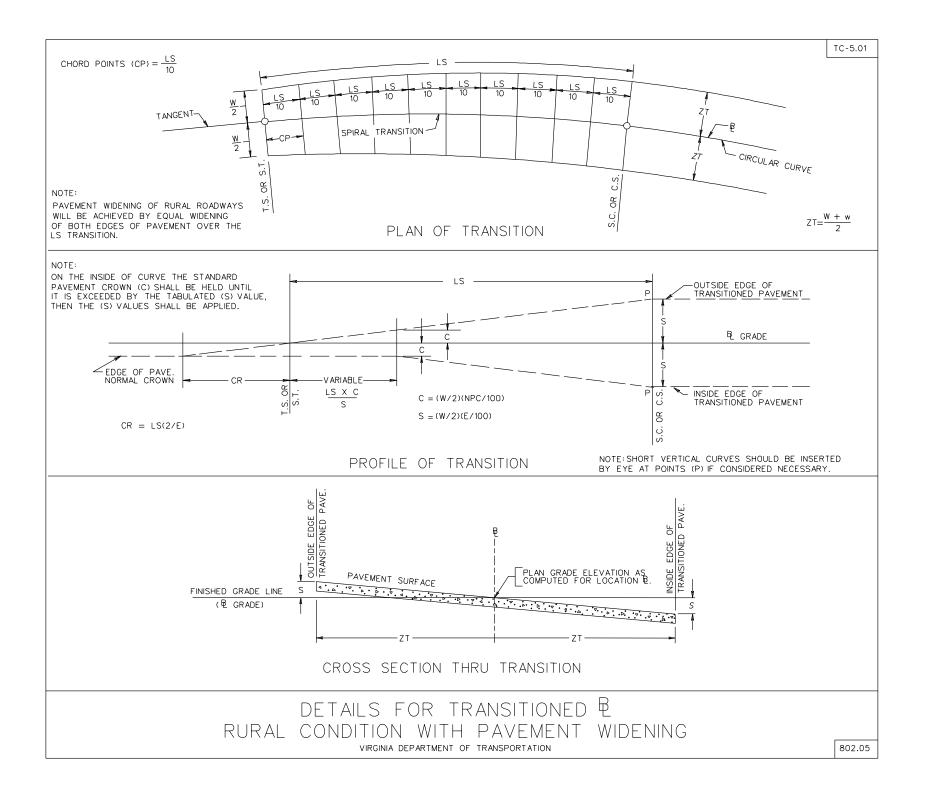
THE DESIGNER SHOULD EXERCISE CAUTION IN THE USE OF COMPOUND AND REVERSE CURVES UNLESS TOPOGRAPHICAL OR RIGHT OF WAY RESTRICTIONS MAKE THEIR USE APPROPIATE. THE USE OF BROKEN-BACK CURVES SHOULD BE AVOIDED EXCEPT WHERE VERY UNUSUAL TOPOGRAPHICAL OR RIGHT OF WAY CONDITIONS MAKE OTHER ALTERNATIVES IMPRACTICAL. THE USE OF BROKEN-BACK CURVES MAY REQUIRE A DESIGN EXCEPTION FROM THE STATE LOCATION AND DESIGN ENGINEER. SEE SHEETS 802.11 THRU 802.14 FOR GENERAL INFORMATION ON COMPOUND, REVERSE AND BROKEN-BACK CURVE INFORMATION. REFER TO APPENDIX A OF THE ROAD DESIGN MANUAL FOR SPECIFIC COMPOUND AND REVERSE CURVE DESIGN INFORMATION.

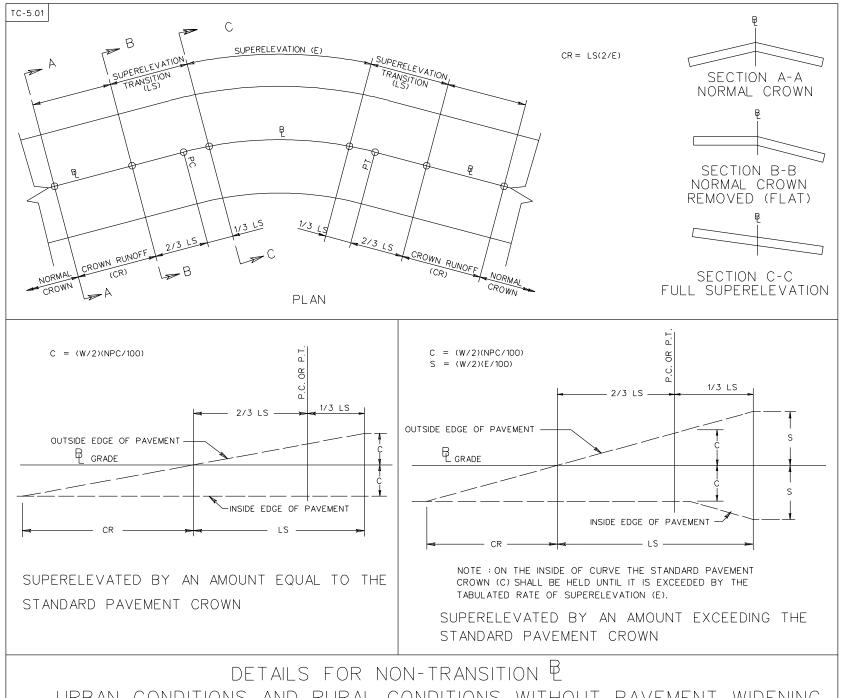
A DESIGN EXCEPTION IS NOT REQUIRED WHEN USING VALUES FROM SHEETS 802.24 THRU 802.44 SINCE THESE TABLES WERE DERIVED WITHIN AASHTO GUIDELINES.

REFER TO APPENDIX A, SECTIONS A-1 AND A-4, OF THE ROAD DESIGN MANUAL FOR INFORMATION ON THE USE OF 18' PAVEMENT WIDTHS (9' LANE WIDTHS).

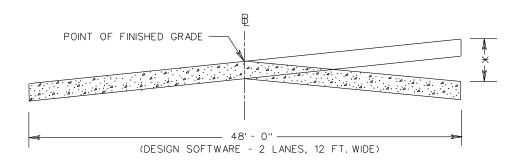
ALL CROWN RUNOFF (CR) VALUES AND TRANSITION LENGTHS (LS) LISTED IN THE TABLES HAVE BEEN ROUNDED UP TO THE NEAREST FOOT. ALL CR VALUES ARE BASED ON A 2% CROWN.

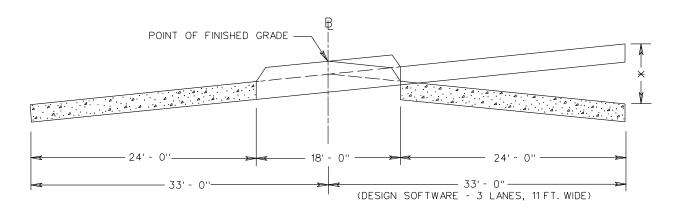
EXPLANATION OF TABLES AND INSTRUCTIONS FOR USE GENERAL CONDITION





URBAN CONDITIONS AND RURAL CONDITIONS WITHOUT PAVEMENT WIDENING VIRGINIA DEPARTMENT OF TRANSPORTATION



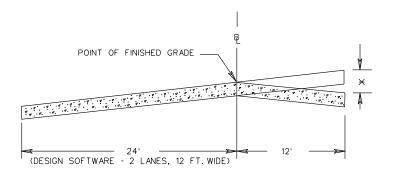


* THE ELEVATION DIFFERENTIAL BETWEEN NORMAL CROWN AND MAXIMUM SUPERELEVATION, RELATIVE TO THE BASELINE PROFILE.

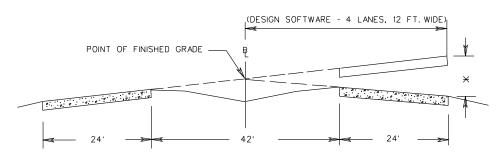
ADDITIONAL INFORMATION MAY BE OBTAINED FROM A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS (AASHTO) BOOK, CHAPTER III - ELEMENTS OF DESIGN (SUPERELEVATION RUNOFF).

ON STANDARD TC-5.01ULS, TC-5.01U , AND TC-5.01R (WITHOUT PAVEMENT WIDENING) SUPERELEVATED CURVES, POSITION THE LS TWO THIRDS (2/3) ON THE TANGENT AND ONE THIRD (1/3) INTO THE CURVE. STATIONS AND ELEVATIONS FOR THESE TRANSITIONS WILL NEED TO BE COMPUTED FOR ALL CHORD POINTS AND SHOWN ON THE PROFILES.

DETAILS OF SUPERELEVATION ABOUT BASELINE



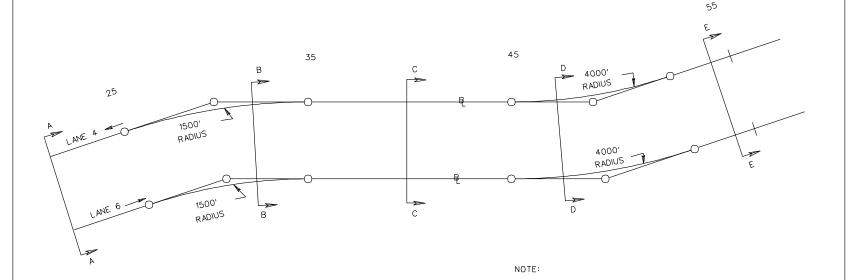
THE PAVEMENT WIDTHS SHOWN IN THE STANDARD TC-5.01 TABLES ON SHEET 802.24 THROUGH 802.44 REPRESENT TWICE THE DISTANCE FROM THE CROWNLINE TO THE EDGE OF PAVEMENT ON THE HIGH SIDE.



* THE ELEVATION DIFFERENTIAL BETWEEN NORMAL CROWN AND MAXIMUM SUPERELEVATION, RELATIVE TO THE BASELINE PROFILE.

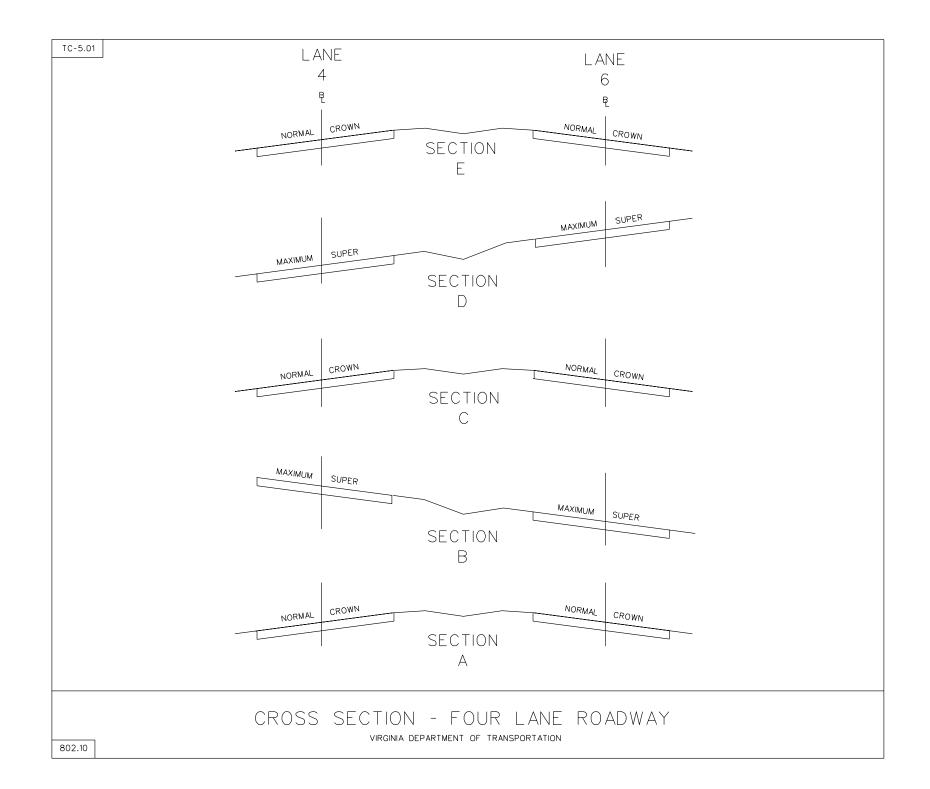
ADDITIONAL INFORMATION MAY BE OBTAINED FROM A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS (AASHTO) BOOK, CHAPTER III - ELEMENTS OF DESIGN (SUPERELEVATION RUNOFF).

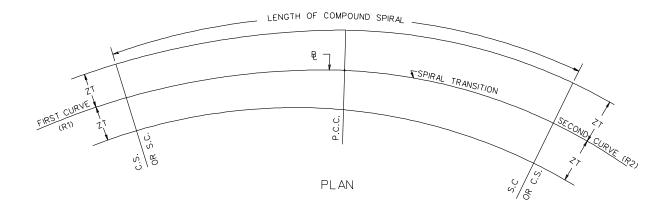
PROJECTS IN WHICH LANES MAY BE ADDED IN THE FUTURE IN THE MEDIAN AREA SHOULD BE DESIGNED WITH THE CONSTRUCTION BASELINE AND POINT OF FINISHED GRADE LOCATED IN THE MIDDLE OF THE MEDIAN. SUPERELEVATION IS TO BE ROTATED FROM THIS BASELINE POINT. THIS WILL PREVENT UNEVEN PAVEMENT PROBLEMS (WHEN ADDITIONAL LANES ARE ADDED IN THE MEDIAN AREA) SUCH AS CROSSOVER GRADES AS WELL AS THE NEED FOR RETAINING WALLS, MEDIAN BARRIERS AND SPECIAL DESIGN DRAINAGE STRUCTURES. ADDITIONAL RIGHT OF WAY OR EASEMENTS, IN MOST SITUATIONS, WILL NOT BE REQUIRED.

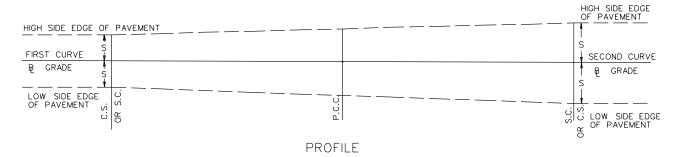


EXAMPLE FOR FOUR LANE ROADWAYS

WHEN $\[mu]$ (Crownline) is on the inside edge of pavement, tangent sections are to be coded as straight.







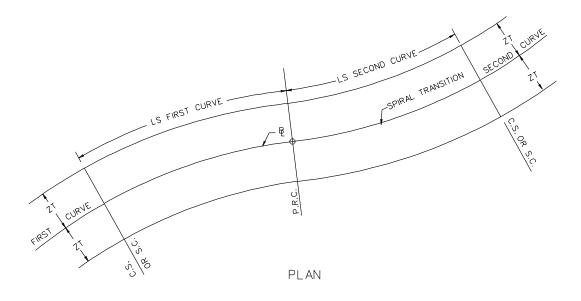
NOTE:

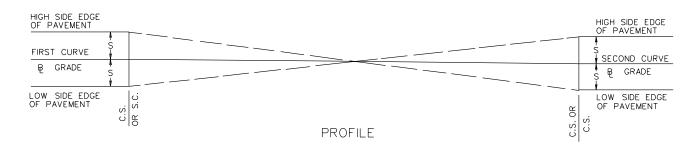
- FOR COMPOUND CURVES ON ROADWAYS, THE RATIO OF FLATTER RADIUS (R1) TO THE SHARPER RADIUS (R2) SHALL NOT EXCEED 1.5:1.
- 2. COMPUTE STRAIGHT LINE WIDENING AND SUPERELEVATION TRANSITION FROM MAXIMUM OF FIRST CURVE TO MAXIMUM OF SECOND CURVE.
- 3. REFER TO APPENDIX A OF THE ROAD DESIGN MANUAL FOR ADDITIONAL COMPOUND CURVE DESIGN INFORMATION.

SPECIFICATION REFERENCE METHOD OF APPLYING TC-5.01 ON COMPOUND CURVES RURAL CONDITIONS WITH PAVEMENT WIDENING

VIRGINIA DEPARTMENT OF TRANSPORTATION

802.11





NOTE.

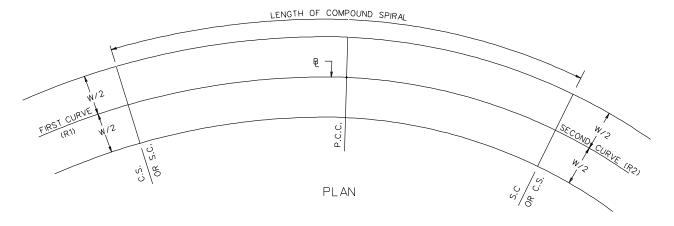
- COMPUTE STRAIGHT LINE WIDENING AND SUPERELEVATION TRANSITION FROM MAXIMUM OF FIRST CURVE TO MAXIMUM OF SECOND CURVE.
- 2. REFER TO APPENDIX A OF THE ROAD DESIGN MANUAL FOR ADDITIONAL REVERSE CURVE DESIGN INFORMATION.

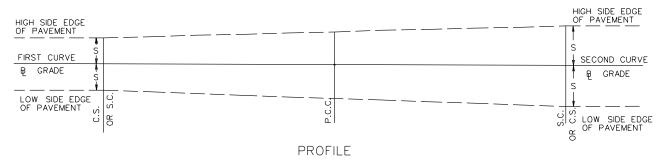
METHOD OF APPLYING TC-5.01 ON REVERSE CURVES RURAL CONDITIONS WITH PAVEMENT WIDENING

VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE



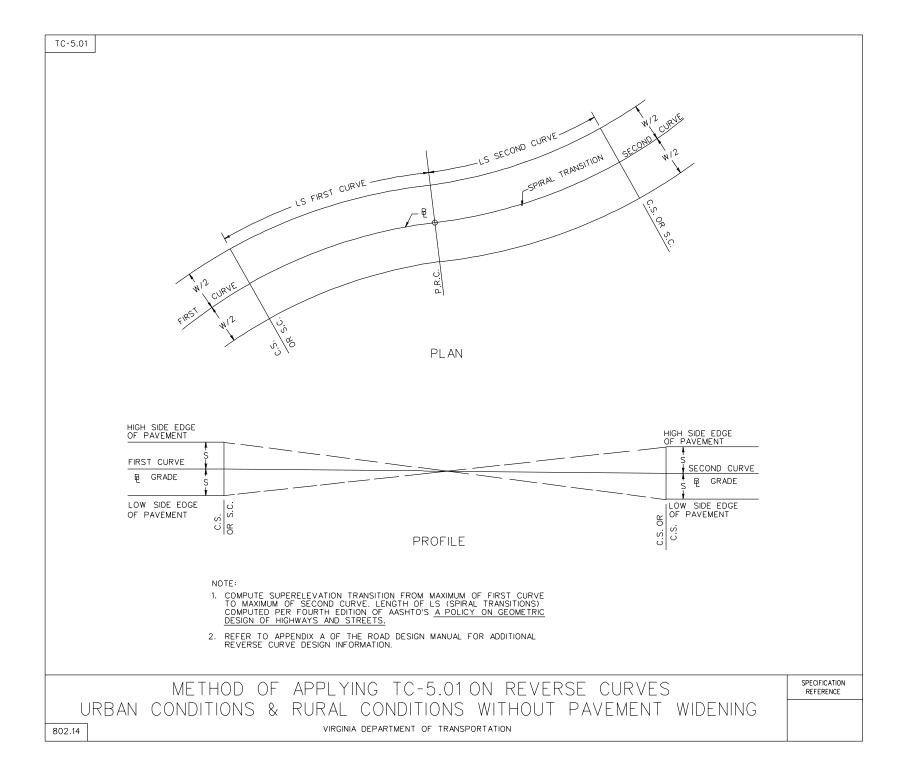




NOTE:

- 1. FOR COMPOUND CURVES ON ROADWAYS, THE RATIO OF FLATTER RADIUS (R1) TO THE SHARPER RADIUS (R2) SHALL NOT EXCEED 1.5:1 WHERE PRACTICAL, A DESIRABLE MAXIMUM RATIO OF 1.75:1 SHOULD BE USED. FOR COMPOUND CURVES ON RAMPS, THE RATIO OF THE FLATTER RADIUS (R1) TO THE SHARPER RADIUS (R2) SHALL NOT EXCEED 2:1.
- 2. COMPUTE SUPERELEVATION TRANSITION FROM MAXIMUM OF FIRST CURVE TO MAXIMUM OF SECOND CURVE. LENGTH OF COMPOUND SPIRAL COMPUTED PER THE FOURTH EDITION OF AASHTO'S A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS.
- 3. REFER TO APPENDIX A OF THE ROAD DESIGN MANUAL FOR ADDITIONAL COMPOUND CURVE DESIGN INFORMATION.

SPECIFICATION REFERENCE METHOD OF APPLYING TC-5.01 ON COMPOUND CURVES URBAN CONDITIONS & RURAL CONDITIONS WITHOUT PAVEMENT WIDENING



		TC-5.01
	THIS SHEET INTENTIONALLY LEFT BLANK	
SPECIFICATION		
REFERENCE		
	VIRGINIA DEPARTMENT OF TRANSPORTATION	802.15

TRANSITION TABLE

LENGTH OF CROWN RUNOFF (CR)	START/END OF TRANSITION (LS)	s	DISTANCE IN FEET TART/END OF TRANS			NORMAL CROWN
	(13)	1	2	3	4	
220	0	44	88	132	176	220
200	0	40	80	120	140	200
180	0	36	72	108	144	180
160	0	32	64	96	128	160
140	0	28	56	84	112	140
120	0	24	48	72	96	120
100	0	20	40	60	80	100
90	0	18	36	54	72	90
80	0	16	32	48	64	80
60	0	15	30	45		60
40	0	20				40

NOTE:

TABLE LISTS POSTIONS ON TRANSITIONS AT WHICH SLOPE STAKES SHOULD BE SET, CONSTRUCTION AND FINAL CROSS-SECTIONS TAKEN, FINE GRADING STAKES (BLUE TOP) SET, AND FORM STAKES SET (CONCRETE PAVEMENT ONLY).

URBAN CONDITIONS RURAL CONDITIONS WITHOUT PAVEMENT WIDENING

FOR USE WITH FLEXIBLE AND CONCRETE PAVEMENT (LS POSITIONED $2/3\pm$ ON TANGENT, $1/3\pm$ ON CURVE)

LENGTH OF	END/ BEGIN CROWN RUNOFF			DISTANCE IN				P.C. OR P.T.		ANCE IN FEET		FULL SUPER ELEVATION (E)
(LS)	(CR)	1	2	3	4	5	6		7	8	9	
480	320	272	224	176	128	80	32	STAKE	16	64	112	160
460	307	261	215	169	123	77	31	STAKE	15	61	107	153
440	293	249	205	161	117	73	29	STAKE	15	59	103	147
420	280	238	196	154	112	70	28	STAKE	14	56	98	140
400	267	227	187	147	107	67	27	STAKE	13	53	93	133
380	253	215	177	139	101	63	25	STAKE	13	51	89	127
360	240	204	168	132	96	60	24	STAKE	12	48	84	120
340	227	193	159	125	91	57	23	STAKE	11	45	79	113
320	213	181	149	117	85	53	21	STAKE	11	43	75	107
300	200	170	140	110	80	50	20	STAKE	10	40	70	100
280	187	159	131	103	75	47	19	STAKE	9	37	65	93
260	173	147 ×	121	95 ×	69	43 ×	17	STAKE *	9	35 ×	61	87
240	160	136 ×	112	88 ×	64	40 ×	16	STAKE *	8	32 ×	56	80
220	147	125 X	103	81 ×	59	37 X	15	STAKE *	7	29 X	51	73
200	133	113 ×	93	73 ×	53	33 ×	13	STAKE X	7	27 ×	47	67
180	120	102 ×	84	66 ×	48	30 ×	12	STAKE *	6	24 ×	42	60
160	107	91 [*]	75	59 ×	43	27 ×	11	STAKE *	5	21 ×	37	53

NOTE :

TABLE GIVING POSITIONS ON CURVES AT WHICH SLOPE STAKES SHOULD BE SET, CONSTRUCTION AND FINAL CROSS-SECTIONS TAKEN, FINE GRADING STAKES (BLUE TOP) SET, AND FORM STAKES SET (CONCRETE PAVEMENT ONLY).

* DENOTES ADDITIONAL STAKING POSITIONS FOR USE WITH CONCRETE PAVEMENT ONLY.

TABLE I

RURAL CONDITIONS WITH PAVEMENT WIDENING

FOR USE WITH FLEXIBLE AND CONCRETE PAVEMENT

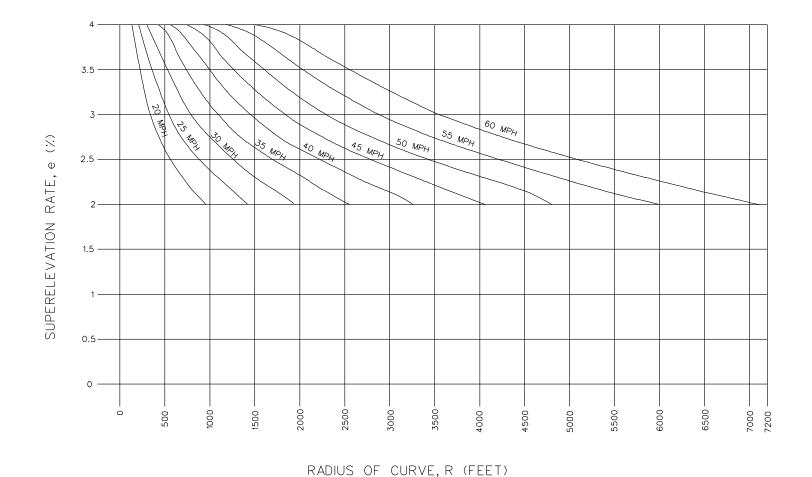
LENGTH OF	T.S. OR S.T.			DISTANCE IN	FEET FROM		т.				S.C. OR C.S.
TRANSITION (LS)		1	2	3	4	5	6	7	8	9	
480	0	48	96	144	192	240	288	336	384	432	480
460	0	46	92	138	184	230	276	322	368	414	460
440	0	44	88	132	176	220	264	308	352	396	440
420	0	42	84	126	168	210	252	294	336	378	420
400	0	40	80	120	160	200	240	280	320	360	400
380	0	38	76	114	152	190	228	266	304	342	380
360	0	36	72	108	144	180	216	252	288	324	360
340	0	34	68	102	136	170	204	238	272	306	340
320	0	32	64	96	128	160	192	224	256	288	320
300	0	30	60	90	120	150	180	210	240	270	300
280	0	28	56	84	112	140	168	196	224	252	280
260	0	26 ×	52	78 ×	104	130 *	156	182 Ж	208	234 ×	260
240	0	24 X	48	72 X	96	120 Ж	144	168 Ж	192	216 ×	240
220	0	22 ×	44	66 X	88	110 *	132	154 ×	176	198 Ж	220
200	0	20 Ж	40	60 ×	80	100 ×	120	140 *	160	180 ×	200
180	0	18 X	36	54 X	72	90 X	108	126 X	144	162 ×	180
160	0	16 🗶	32	48 X	64	80 ×	96	112 *	128	144 *	160

NOTE :

TABLE GIVING POSITIONS ON TRANSITION CURVES AT WHICH SLOPE STAKES SHOULD BE SET, CONSTRUCTION AND FINAL CROSS-SECTIONS TAKEN, FINE GRADING STAKES (BLUE TOP) SET, AND FORM STAKES SET (CONCRETE PAVEMENT ONLY).

* DENOTES ADDITIONAL STAKING POSITIONS FOR USE WITH CONCRETE PAVEMENT ONLY.

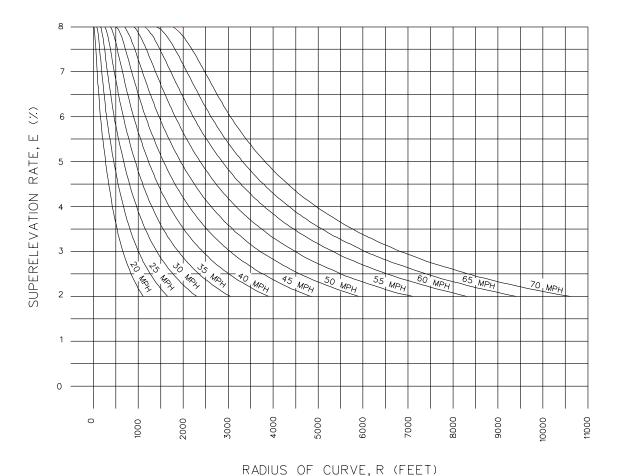
TABLE 2



DESIGN SUPERELEVATION RATES URBAN CONDITIONS

VIRGINIA DEPARTMENT OF TRANSPORTATION

802.19



DESIGN SUPERELEVATION RATES RURAL CONDITIONS VIRGINIA DEPARTMENT OF TRANSPORTATION

LEGEND

- C- RATE OF CHANGE OF SIDE FRICTION (f) IN FT./SEC.
- e- SUPERELEVATION RATE.
- f- FRICTION FACTOR.
- LS- LENGTH OF SUPERELEVATION TRANSITION.
- R- RADIUS OF CURVE.
- DV- DESIGN VELOCITY UTILIZING SUPERELEVATION.
- NC- MAXIMUM VELOCITY WITH NO SUPERELEVATION (NORMAL CROWN).

URBAN LOW SPEED DESIGN TABLE

DV/NC (MPH)	MAX. f	С	MIN. LS (FEET)
45	0.161	2.75	115
40	0.178	3.00	100
35	0.197	3.25	90
30	0.221	3.50	90
25	0.252	3.75	90
20	0.300	4.00	90

FRICTION FACTORS (f) FOR ODD VELOCITIES NOT LISTED SHOULD BE DERIVED BY INTERPOLATION.

FOR LS LENGTHS FOR INTERMEDIATE VELOCITIES NOT LISTED IN TABLE USE THE LS FOR NEAREST VELOCITY IN TABLE.

GENERAL DESIGN CONSIDERATIONS

- 1. WHEN "URBAN LOW SPEED" DESIGNS UTILIZE SUPERELEVATION, THEY WILL BE SUPERELEVATED BY AN AMOUNT EQUAL TO THE NORMAL CROWN (TYPICALLY 2.0%) AND THE APPROXIMATE MAXIMUM SAFE SPEED (DV) AFFORDED THEREBY.
- WHEN "URBAN LOW SPEED DESIGN" WITH NO SUPERELEVATION, THE APPROXIMATE MAXIMUM SAFE SPEED (NC) IS CALCULATED USING A NEGATIVE NORMAL CROWN (TYPICALLY -2.0 %).
- 3. WHEN THE CURVE IS SUPERELEVATED, THE LS IS APPLIED IN THE SAME MANNER AS IN URBAN CONDITIONS WITH THE CROWN RUNOFF (CR) BEING EQUAL TO THE LS VALUE. THE CROWN RUNOFF (CR) IS ALWAYS ACHIEVED OUTSIDE OF THE TRANSITION (LS).
- 4. PLEASE NOTE THAT THE RADIUS VALUES LISTED ON PAGE 802.24 HAVE BEEN ROUNDED UP TO THE NEAREST FOOT.

EXAMPLES

DV = 21 mph

e = +2.0 %

f = MAX f = INTERPOLATED DIFFERENCE BETWEEN LISTED FRICTION FACTORS

f = 0.300-[1/5(0.300-0.252)]=0.2904 (ROUND TO 0.29)

LS = 47.2 f DV/C

LS = 47.2(0.29)(21)/4=71.862 FT.

71.862 <90 THEREFORE LS*90 FT.

Rmin. $= DV^2/15(e+f)$

Rmin. = (21) / 15(0.02 + 0.29) = 94.83870968 FT.

NC - 37 mph

e = -2.0 %

f = MAX f = INTERPOLATED DIFFERENCE BETWEEN LISTED FRICTION FACTORS

f = 0.197-[2/5(0.197-0.178)]=0.1894 (ROUND TO 0.189)

Rmin. = NC $^{2}/15(-e + f)$

Rmin. = $(37)^2 / 15(-0.02 + 0.189) = 540.0394477$ FT.

METHODOLOGIES FOR CALCULATING TC-5.01 VALUES FOR URBAN LOW-SPEED STREETS

CURVE WIDENING TABLES

SU DESIGN VEHICLE

COMPONENT	SIZE
OVERALL WIDTH (u)	8.0 ft
WHEELBASE (L)	20 ft
FRONT OVERHANG (A)	4 ft

LATERAL CLEARANCE

LANE WIDTH	CLEARANCE (C)
9 ft	1.5 ft
10 ft	2 ft
11 ft	2.5 ft
12 ft	3 ft
16 ft	5 ft

ADJUSTMENT FACTORS

NUMBER OF LANES ROTATED	ADJUSTMENT FACTOR (bw)
1	1.00
1.5	0.8333
2	0.75
2.5	0.70
3	0.6667
3.5	0.6425

RELATIVE GRADIENTS

DESIGN SPEED VD MPH	MAXIMUM RELATIVE GRADIENT (rg)	MIN. TRANSITION LENGTH IN FEET RURAL CONDITIONS WITH PAVEMENT WIDENING AND REVERSE CURVES FOR ALL CONDITIONS							
		URBAN	RURAL						
20	0.74	100	60						
25	0.70	100	80						
30	0.66	100	100						
35	0.62	120	120						
40	0.58	120	120						
45	0.54	140	140						
50	0.50	160	160						
55	0.47	180	180						
60	0.45	180	180						
65	0.43	200	200						
70	0.40	220	220						

- A FRONT OVERHANG OF DESIGN VEHICLE FROM APPROPRIATE TABLE.
- bw ADJUSTMENT FACTOR FROM TABLE.
- C LATERAL CLEARANCE OF DESIGN VEHICLE FROM APPROPRIATE TABLE.
- E SUPERELEVATION RATE FROM APPROPRIATE TABLE.
- ${\sf F_A}$ CALCULATED WIDTH OF OVERHANG FOR DESIGN VEHICLE.
- L WHEELBASE OF DESIGN VEHICLE FROM APPROPRIATE TABLE.
- LS LENGTH OF SPIRAL OR SUPERELEVATION TRANSITION LENGTH.

- DEFINITIONS
- M MULTIPLE LANE FACTOR.
- N NUMBER OF LANES.
- n₁- NUMBER OF LANES ROTATED (FROM TABLES).
- Pw PAVEMENT WIDTH.
- R RADIUS OF CURVE.
- rg RELATIVE GRADIENT FROM APPROPRIATE TABLE.
- U CALCULATED TRACK WIDTH OF DESIGN VEHICLE.

- u TRACK WIDTH OF DESIGN VEHICLE FROM APPROPRIATE TABLE.
- VD DESIGN VELOCITY.
- w CALCULATED WIDENING.
- W PAVEMENT WIDTH
- W_C CALCULATED TOTAL CURVE WIDTH.
- Wn WIDTH OF LANE.
- Z CALCULATED EXTRA WIDTH ALLOWANCE.

GENERAL DESIGN CONSIDERATIONS

- WHERE PAVEMENT WIDENING IS REQUIRED, THE APPROPRIATE WIDENING IS ADDED TO THE LANE WIDTH WHEN CALCULATING THE TRANSITION LENGTH (LS).
- 2. THE COMPUTED TRANSITION LENGTH (LS) IS ROUNDED UP TO THE NEAREST FOOT.
- 3. WHEN THE TRANSITION LENGTH (LS) IS CALCULATED, IT MUST BE COMPARED WITH THE MINIMUM VALUE LISTED IN THE APPROPRIATE COLUMN ON THE RELATIVE
- 4. CROWN RUNOFF IS ALWAYS ACHIEVED OUTSIDE OF THE TRANSITION.
- 5. NO PAVEMENT WIDENING IS REQUIRED FOR URBAN ROADWAYS.
- 6. NO PAVEMENT WIDENING IS REQUIRED FOR RURAL ROADWAYS WITH A CURVE RADIUS GREATER THAN 2865 FEET.

- NO PAVEMENT WIDENING IS REQUIRED FOR RURAL ROADWAYS WITH 12 FOOT WIDE LANES AND A CURVE RADIUS GREATER THAN 881 FEET.
- 8. PAVEMENT WIDENING IS APPLIED ONLY WHEN CALCULATED WIDENING (w) IS EQUAL TO OR GREATER THAN 2 FEET.
- 9. WHEN CALCULATING WIDENING (w) FOR MULTI-LANE RURAL ROADWAYS, WIDENING IS FIRST CALCULATED USING THE SINGLE LANE WIDTH FOR "W".
- 10. AN ALTERNATE METHOD FOR MULTI-LANE UNDIVIDED PAVEMENTS (48'). THE LS IS 1.5 TIMES (M-1.5) THE CORRESPONDING LENGTH FOR TWO LANE HIGHWAYS; AND FOR SIX LANE UNDIVIDED PAVEMENTS (72'), THE LS IS TWO TIMES (M-2) THE CORRESPONDING LENGTH FOR TWO LANE HIGHWAYS.
- 11. CALCULATED WIDENING IS ROUNDED UP TO THE NEAREST 0.1 FOOT.

NO WIDENING REQUIRED FORMULAS USED TO CALCULATE TRANSITION LENGTH (LS) AND WIDENING (W)

 $LS = b_w(W_n E/rg)$

LS = M(WE/rg) (ALT. MULTI-LANE)

WIDENING REQUIRED

 $LS = b_w[E n_1(W_n + w/N)/rg]$ LS = m[E(W + w/N)/rg] (ALT. MULTI-LANE) $U = u + R - \sqrt{R^2 - L^2}$

 $F_{A} = \sqrt{R^2 + A(2L + A)} - R$

 $Z = (V_D / \sqrt{R})$

 $w = W_C - 2W_D$

 $W_C = N(U + C) + F_{\Delta} + Z$

FOR SOLVED PROBLEMS USING THIS METHODOLOGY, SEE THE EXAMPLES ON PAGE 802.23

METHODOLOGIES FOR CALCULATING TC-5.01 VALUES

RURAL EXAMPLE 20 FT PAVEMENT WIDTH (DESIGN SOFTWARE - 1 LANE AT 10 FT)

R = 1000 FT $V_D = 50 MPH$ $W_n = 10 \text{ FT}$ rg = 0.50E = 7.6 (7.6% PER 802.40)

 $U = u + R - \sqrt{R^2 - L^2}$ $U = 8.0 + 1000 - \sqrt{(1000)^2 - (20)^2}$ U = 8.20002

 $F_{\Delta} = \sqrt{R^2 + A(2L + A)} - R$ $F_A = \sqrt{(1000)^2 + 4[2(20) + 4] - 1000}$ $F_{A} = .087996$

 $Z = (V_D / \sqrt{R})$ $Z = (50 / \sqrt{1000})$ Z = 1.58

 $W_C = N (U + C) + F_A + Z$ $W_{C} = 2(8.20002 + 2) + 0.087996 + 1.58$ $W_{C} = 22.0680$

 $W = W_C - 2W_0 = 22.0680 - 2(10) = 2.1$

(R<2865 & w>2 THEREFORE WIDENING IS REQUIRED) $LS = [E n_{s}(W_{n} + w/2)/ rq]b_{w}$ LS = [7.6(1)(10 + 2.1/2) / 0.50] 1LS = 7.6 (11.05)/0.50LS = 167.96

RURAL EXAMPLE 72 FT PAVEMENT WIDTH (DESIGN SOFTWARE - 3 LANES AT 12 FT)

R = 500 FT $V_D = 40 \text{ MPH}$ $V_D = 40 \text{ M/H}$ $W_n = 12 \text{ FT}$ rg = 0.58E = 8.0 (8% PER PAGE 802.38)

 $U = u + R - \sqrt{R^2 - L^2}$ $U = 8.0 + 500 - \sqrt{(500)^2 - (20)^2}$ U = 8.4002

 $F_{\Delta} = \sqrt{R^2 + A(2L + A)} - R$ $F_A = \sqrt{(500)^2 + 4[2(20) + 4] - 500}$ $F_A = .1760$

 $Z = (V_D / \sqrt{R})$ $Z = (40 / \sqrt{500})$ Z = 1.7885

 $W_C = 2 (U + C) + F_A + Z$ $W_C = 2(8.4002 + 3.0) + .1760 + 1.7885$ $W_{C} = 24.7651$

 $w = W_C - 2W_D = 24.7651 - 2(12) = 0.7651(0.8)$

FOR 72' PAVEMENT WIDTH w = 3(0.8) = 2.4

(R<881 & w>2 THEREFORE WIDENING IS REQUIRED) $LS = [E \ n_1 (W_n + w/3)/ \ rg] b_w$ LS = [8 (3) (12 + 2.4/3) / 0.58] 0.6667LS = (307.2/0.58) 0.6667

LS = 353.1211

 $LS = M[E(W_n + w/N)/rg]$ LS = 2 [8(12 + 4.5/3) / 0.58]

LS = 2 (102.4/0.58)LS = 353.1034

URBAN EXAMPLES

24 FT PAVEMENT WIDTH (DESIGN SOFTWARE - 1 LANE AT 12 FT)

 $V_D = 40 \text{ MPH}$ R = 600 FT $W_n^- = 12 \text{ FT}$ rg = 0.58E = 4.0 (4% PER PAGE 802.29)

 $LS = (W_n n, E/rg) b_w$ LS = [12(1)(4)/0.58] 1.00LS = (48/.058)LS = 82.7586

66 FT PAVEMENT WIDTH
(DESIGN SOFTWARE - 3 LANES AT 11 FT)

 $V_D = 40 MPH$ R = 600 FT $W_n = 11 FT$ rg = 0.58E = 4.0 (4% PER PAGE 801.29)

 $LS = b_w (W_n n_1 E/rg)$ LS = 0.6667 [11(3)(4)/ 0.58]LS = 0.6667 (132/0.58)LS = 151.7317

 $LS = M (E W_n/rq)$ LS = 2 [4(11)/ 0.58]LS = 2 (44/0.58)

LS = 151.7241

CALCULATED TC-5.01 EXAMPLES

MINIMUM RADII AND TRANSITION LENGTHS FOR 2% SUPERELEVATION

					LS (FEET)
RADIUS	E	F	DV		PAVEMENT WIDTH (W)
(FEET)	(%)		(MPH)	W <u><</u> 72 FT.	W > 72 FT
>738	2.0	.163	45	126	
539	2.0	.178	40	113	NOTF:
377	2.0	.197	35	101	FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED
249	2.0	.221	30	90	BY THE DESIGN SOFTWARE.
154	2.0	.252	25	80	
84	2.0	.300	20	75	

MINIMUM RADIIFOR DESIGNS UTILIZING NORMAL PAVEMENT CROWN

RADIUS (FEET)	F	NC (MPH)
> 945	.163	45
676	.178	40
462	.197	35
299	.221	30
180	.252	25
96	.300	20

FT 66 FT	72								
(FEET) (%) DESIGN SOFTWARE EQUIVALENTS (NUMBER OF LANES AT LANE WIDTH									
IO' 3 6 11'	3 @								
,	10' 3 @ 11'								

FΤ @ 12' CR LS CR CR LS CR CR LS LS LS LS NC Ω 2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E. CR. AND LS VALUES.

SPECIFICATION REFERENCE

TRANSITION CURVES - URBAN 20 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

802.25

TC-5.01 DESIGN FACTORS FOR A DESIGN SPEED OF 25 MPH (URBAN) USING E= 4% MAX.													
						MENT W		. , , , ,					
RADIUS	E	24	24 FT 36 FT 48 FT 60 FT 66 FT								72	FT	
(FEET)	(%)	DE	ESIGN S	SOF TWA	RE EQL	JIVALEN	TS (NUI	MBER O	F LANE	S AT L	ANE WII	LTH)	
		1 @ 12' 1.5 @ 12'		2 @	12' 3 @ 10'		3 @ 11'		3 @ 12'				
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS
2500	NC	0	0	0	0	0	0	0	0	0	0	0	0
1407	2.0	35	35	43	43	52	52	58	58	63	63	69	69
1299	2.1	35	36	43	45	52	54	58	60	63	66	69	72
1195	2.2	35	38	43	48	52	57	58	63	63	70	69	76
1094	2.3	35	40	43	50	52	60	58	66	63	73	69	79
990	2.4	35	42	43	52	52	62	58	69	63	76	69	83
883	2.5	35	43	43	54	52	65	58	72	63	79	69	86
793	2.6	35	45	43	56	52	67	58	75	63	82	69	90
718	2.7	35	47	43	58	52	70	58	78	63	85	69	93
654	2.8	35	48	43	60	52	72	58	80	63	88	69	96
598	2.9	35	50	43	63	52	75	58	83	63	92	69	100
548	3.0	35	52	43	65	52	78	58	86	63	95	69	103
505	3.1	35	54	43	67	52	80	58	89	63	98	69	107
466	3.2	35	55	43	69	52	83	58	92	63	101	69	110
430	3.3	35	57	43	71	52	85	58	95	63	104	69	114
397	3.4	35	59	43	73	52	88	58	98	63	107	69	117
367	3.5	35	60	43	75	52	90	58	100	63	110	69	120
339	3.6	35	62	43	78	52	93	58	103	63	114	69	124
311	3.7	35	64	43	80	52	96	58	106	63	117	69	127
284	3.8	35	66	43	82	52	98	58	109	63	120	69	131
255	3.9	35	67	43	84	52	101	58	112	63	123	69	134

52

103

58

115

63

126

69

138

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

4.0

35

43

69

DESIGN FACTORS FOR A DESIGN SPEED OF 30 MPH (URBAN) USING E= 4% MAX.

CONDINIO E 17. W///C														
					PAVE	MENT V	NIDTH							
RADIUS	E	24	FT	36	FT	48	FT	60	FT	66 FT 72 FT				
(FEET)	(%)		ESIGN	SOFTW	ARE EQ	UIVALEN	JIVALENTS (NUMBER OF LANES AT LANE WIDTH)							
		1@	12'	1.5 @	2 12'	2 @	2 @ 12'		10'	3 @	11'	3 @ 12'		
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	
3000	NC	0	0	0	0	0	0	0	0	0	0	0	0	
1940	2.0	37	37	46	46	55	55	61	61	67	67	73	73	
1795	2.1	37	39	46	48	55	58	61	64	67	70	73	77	
1658	2.2	37	40	46	50	55	60	61	67	67	74	73	80	
1525	2.3	37	42	46	53	55	63	61	70	67	77	73	84	
1393	2.4	37	44	46	55	55	66	61	73	67	80	73	88	
1255	2.5	37	46	46	57	55	69	61	76	67	84	73	91	
1134	2.6	37	48	46	60	55	71	61	79	67	87	73	95	
1030	2.7	37	50	46	62	55	74	61	82	67	90	73	99	
941	2.8	37	51	46	64	55	77	61	85	67	94	73	102	
863	2.9	37	53	46	66	55	80	61	88	67	97	73	106	
794	3.0	37	55	46	69	55	82	61	91	67	100	73	110	
732	3.1	37	57	46	71	55	85	61	94	67	104	73	113	
677	3.2	37	59	46	73	55	88	61	97	67	107	73	117	
627	3.3	37	60	46	75	55	90	61	100	67	110	73	120	
580	3.4	37	62	46	78	55	93	61	104	67	114	73	124	
537	3.5	37	64	46	80	55	96	61	107	67	117	73	128	
496	3.6	37	66	46	82	55	99	61	110	67	120	73	131	
457	3.7	37	68	46	85	55	101	61	113	67	124	73	135	
417	3.8	37	70	46	87	55	104	61	116	67	127	73	139	
375	3.9	37	71	46	89	55	107	61	119	67	130	73	142	
300	4.0	37	73	46	91	55	110	61	122	67	134	73	146	

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

TRANSITION CURVES - URBAN 30 MPH DESIGN SPEED

TC-5.01	TC-5.01 DESIGN FACTORS FOR A DESIGN SPEED OF 35 MPH (URBAN) USING E= 4% MAX.												
						MENT W							
RADIUS	E	24 FT 36 FT 48 FT						60 FT 66 FT 72 F					FT
(FEET)	(%)	D	ESIGN	SOFTWA	ARE EQI	UIVALEN	ITS (NL	IMBER (OF LANE	S AT LANE WIDTH)			
		1 @ 12' 1.5 @ 12'			2 @	12'	3 @	10'	3 @	11'	3 @ 12'		
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS
4000	NC	0	0	0	0	0	0	0	0	0	0	0	0
2561	2.0	39	39	49	49	59	59	65	65	71	71	78	78
2374	2.1	39	41	49	51	59	61	65	68	71	75	78	82
2199	2.2	39	43	49	54	59	64	65	71	71	79	78	86
2031	2.3	39	45	49	56	59	67	65	75	71	82	78	90
1866	2.4	39	47	49	59	59	70	65	78	71	86	78	93
1697	2.5	39	49	49	61	59	73	65	81	71	89	78	97
1538	2.6	39	51	49	63	59	76	65	84	71	93	78	101
1403	2.7	39	53	49	66	59	79	65	88	71	96	78	105
1285	2.8	39	55	49	68	59	82	65	91	71	100	78	109
1182	2.9	39	57	49	71	59	85	65	94	71	103	78	113
1090	3.0	39	59	49	73	59	88	65	97	71	107	78	117
1008	3.1	39	60	49	75	59	90	65	100	71	110	78	120
933	3.2	39	62	49	78	59	93	65	104	71	114	78	124
865	3.3	39	64	49	80	59	96	65	107	71	118	78	128
802	3.4	39	66	49	83	59	99	65	110	71	121	78	132
743	3.5	39	68	49	85	59	102	65	113	71	125	78	136
688	3.6	39	70	49	88	59	105	65	117	71	128	78	140
634	3.7	39	72	49	90	59	108	65	120	71	132	78	144
580	3.8	39	74	49	92	59	111	65	123	71	135	78	148
522	3.9	39	76	49	95	59	114	65	126	71	139	78	151
420	4.0	39	78	49	97	59	117	65	130	71	142	78	155

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

TRANSITION CURVES - URBAN 35 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

802.28

DESIGN FACTORS FOR A DESIGN SPEED OF 40 MPH (URBAN) USING E= 4% MAX.

CONDAIN USING L- +/. WAX.													
				PAVE	MENT W	VIDTH							
E	24	FT	36	FT	48	FT	60	FT	66 FT 72 FT				
(%)	DESI	GN SOF	TWARE	EQUIV	ALENTS	LENTS (NUMBER OF LANES AT LANE WIDTH)							
	1@	12'	1.5 @	12'	2 @	12'	3 @	10'	3 @ 11'		3 @ 12'		
	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	
NC	0	0	0	0	0	0	0	0	0	0	0	0	
2.0	42	42	52	52	63	63	69	69	76	76	83	83	
2.1	42	44	52	55	63	66	69	73	76	80	83	87	
2.2	42	46	52	57	63	69	69	76	76	84	83	92	
2.3	42	48	52	60	63	72	69	80	76	88	83	96	
2.4	42	50	52	63	63	75	69	83	76	92	83	100	
2.5	42	52	52	65	63	78	69	87	76	95	83	104	
2.6	42	54	52	68	63	81	69	90	76	99	83	108	
2.7	42	56	52	70	63	84	69	94	76	103	83	112	
2.8	42	58	52	73	63	87	69	97	76	107	83	116	
2.9	42	60	52	75	63	90	69	100	76	110	83	120	
3.0	42	63	52	78	63	94	69	104	76	114	83	125	
3.1	42	65	52	81	63	97	69	107	76	118	83	129	
3.2	42	67	52	83	63	100	69	111	76	122	83	133	
3.3	42	69	52	86	63	103	69	114	76	126	83	137	
3.4	42	71	52	88	63	106	69	118	76	129	83	141	
3.5	42	73	52	91	63	109	69	121	76	133	83	145	
3.6	42	75	52	94	63	112	69	125	76	137	83	149	
3.7	42	77	52	96	63	115	69	128	76	141	83	154	
3.8	42	79	52	99	63	118	69	132	76	145	83	158	
3.9	42	81	52	101	63	122	69	135	76	148	83	162	
4.0	42	83	52	104	63	125	69	138	76	152	83	166	
	NC 2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9	(%) DESN 1 @ CR NC 0 2.0 42 2.1 42 2.2 42 2.3 42 2.4 42 2.5 42 2.6 42 2.7 42 2.8 42 2.9 42 3.0 42 3.1 42 3.2 42 3.3 42 3.4 42 3.5 42 3.6 42 3.7 42 3.6 42 3.7 42 3.8 42 3.9 42 3.9 42	DESIGN SOF 1 @ 12' CR	E 24 FT 36 (%) DESIGN SOFTWARE 1 e 12' 1.5 e CR LS CR NC 0 0 0 2.0 42 42 52 2.1 42 44 52 2.2 42 46 52 2.3 42 48 52 2.4 42 50 52 2.5 42 52 52 2.6 42 54 52 2.7 42 56 52 2.8 42 58 52 2.9 42 60 52 3.0 42 63 52 3.1 42 65 52 3.2 42 67 52 3.3 42 69 52 3.4 42 71 52 3.6 42 75 52 3.7 42 77 52 3.8 42 79 52 3.9 42 81 52	E	E 24 FT 36 FT 48 (%) DESIGN SOFTWARE EQUIVALENTS 1 e 12' 1.5 e 12' 2 e CR LS CR LS CR NC 0 0 0 0 0 0 2.0 42 44 52 52 55 63 2.1 42 44 52 55 55 63 2.2 42 46 52 57 63 2.3 42 48 52 60 63 2.4 42 50 52 63 63 2.5 42 50 52 63 63 2.6 42 54 52 66 63 2.7 42 56 52 70 63 2.8 42 58 52 73 63 2.9 42 60 52 75 63 3.0 42 63 52 78 63 3.1 42 65 52 81 63 3.2 42 67 52 83 63 3.3 42 69 52 86 63 3.4 42 71 52 88 63 3.5 42 73 52 91 63 3.6 42 75 52 94 63 3.7 42 77 52 96 63 3.8 42 79 52 99 63 3.9 42 81 52 101 63	E PAVEMENT WIDTH (%) DESIGN SOFTWARE EQUIVALENTS (NUMBI 1 to 12') 1.5 € 12' 2 € 12' CR LS 42 44 52 55 63 69 2.3 42 48 52 55 63 69 2.3 42 48 52 60 63 72 2.4 42 50 52 63 63 75 2.5 42 52 52 65 63 78 2.5 42 52 52 65 63 81 2.7 42 56 52	E	E	E	E PAVEMBRY WIDTH	PAVENCE PAV	

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

DESIGN FACTORS FOR A DESIGN SPEED OF 45 MPH (URBAN) USING E= 4% MAX.

							PAVE	MENT W	IDTH					
RADIUS	E	24	24 FT 36 FT			48 F	48 FT		60 FT		66 FT		72 FT	
(FEET)	(%)			DESIGN SOFTWARE EQUIVALENTS (NUMBER OF LANES AT LANE WIDTH)									1)	
		1 @ 12'		1.5 @ 12'		2 @ 12'		3 @ 10'		3 @ 11'		3 @	12'	
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	
6000	NC	0	0	0	0	0	0	0	0	0	0	0	0	
4076	2.0	45	45	56	56	67	67	75	75	82	82	89	89	
3790	2.1	45	47	56	59	67	70	75	78	82	86	89	94	
3523	2.2	45	49	56	62	67	74	75	82	82	90	89	98	
3271	2.3	45	52	56	64	67	77	75	86	82	94	89	103	
3029	2.4	45	54	56	67	67	80	75	89	82	98	89	107	
2790	2.5	45	56	56	70	67	84	75	93	82	102	89	112	
2552	2.6	45	58	56	73	67	87	75	97	82	106	89	116	
2341	2.7	45	60	56	75	67	90	75	100	82	110	89	120	
2155	2.8	45	63	56	78	67	94	75	104	82	115	89	125	
1990	2.9	45	65	56	81	67	97	75	108	82	119	89	129	
1843	3.0	45	67	56	84	67	100	75	112	82	123	89	134	
1710	3.1	45	69	56	87	67	104	75	115	82	127	89	138	
1589	3.2	45	72	56	89	67	107	75	119	82	131	89	143	
1477	3.3	45	74	56	92	67	110	75	123	82	135	89	147	
1374	3.4	45	76	56	95	67	114	75	126	82	139	89	152	
1276	3.5	45	78	56	98	67	117	75	130	82	143	89	156	
1184	3.6	45	80	56	100	67	120	75	134	82	147	89	160	
1093	3.7	45	83	56	103	67	124	75	138	82	151	89	165	
1003	3.8	45	85	56	106	67	127	75	141	82	155	89	169	
905	3.9	45	87	56	109	67	130	75	145	82	159	89	174	
730	4.0	45	89	56	112	67	134	75	149	82	163	89	178	

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

DESIGN FACTORS FOR A DESIGN SPEED OF 50 MPH (URBAN) USING E= 4 % MAX.

							PAVE	MENT	WIDTH				
RADIUS	E	24	FT	36	FT	48	FT	60	FT	66	FT	72	FT
(FEET)	(%)	DE	SIGN	SOFTW.	ARE EC	UIVALE	NTS (N	UMBER	OF LA	NES A	T LAN	WIDT	H)
		1 @	12'	1.5	⊉ 12'	2 @	12'	3 @	10'	3 @	2 11'	3 @	12'
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS
8000	NC	0	0	0	0	0	0	0	0	0	0	0	0
4792	2.0	48	48	60	60	72	72	80	80	88	88	96	96
4629	2.1	48	51	60	63	72	76	80	84	88	93	96	101
4310	2.2	48	53	60	66	72	80	80	88	88	97	96	106
4010	2.3	48	56	60	69	72	83	80	92	88	102	96	111
3723	2.4	48	58	60	72	72	87	80	96	88	106	96	116
3444	2.5	48	60	60	75	72	90	80	100	88	110	96	120
3166	2.6	48	63	60	78	72	94	80	104	88	115	96	125
2911	2.7	48	65	60	81	72	98	80	108	88	119	96	130
2686	2.8	48	68	60	84	72	101	80	112	88	124	96	135
2486	2.9	48	70	60	87	72	105	80	116	88	128	96	140
2306	3.0	48	72	60	90	72	108	80	120	88	132	96	144
2143	3.1	48	75	60	93	72	112	80	124	88	137	96	149
1994	3.2	48	77	60	96	72	116	80	128	88	141	96	154
1857	3.3	48	80	60	99	72	119	80	132	88	146	96	159
1729	3.4	48	82	60	102	72	123	80	136	88	150	96	164
1608	3.5	48	84	60	105	72	126	80	140	88	154	96	168
1493	3.6	48	87	60	108	72	130	80	144	88	159	96	173
1381	3.7	48	89	60	111	72	134	80	148	88	163	96	178
1268	3.8	48	92	60	114	72	137	80	152	88	168	96	183
1146	3.9	48	94	60	117	72	141	80	156	88	172	96	188
929	4.0	48	96	60	120	72	144	80	160	88	176	96	192

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR AND LS VALUES.

TRANSITION CURVES - URBAN 50 MPH DESIGN SPEED

TC-5.01

DESIGN FACTORS FOR A DESIGN SPEED OF 55 MPH (URBAN) USING E= 4% MAX.

				VOIN	DAIN	031110) L-	+/, IVI	AA.				
							PAVE	MENT W	IDTH				
RADIUS	E	24	FT	36 F	Г	48 F	Т	60 F	Т	66 F	Т	72 F	Т
(FEET)	(%)		DE	SIGN SO	FTWARE	EQUIVA	LENTS (NUMBER	OF LAN	NES AT	LANE W	(IDTH)	
		1@	12'	1.5 @	12'	2 @	12'	3 @	10'	3 @	11'	3 @	12'
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS
10000	NC	0	0	0	0	0	0	0	0	0	0	0	0
5995	2.0	52	52	64	64	77	77	86	86	94	94	103	103
5592	2.1	52	54	64	68	77	81	86	90	94	99	103	108
5218	2.2	52	57	64	71	77	85	86	94	94	103	103	113
4869	2.3	52	59	64	74	77	89	86	98	94	108	103	118
4538	2.4	52	62	64	77	77	92	86	103	94	113	103	123
4220	2.5	52	64	64	80	77	96	86	107	94	118	103	128
3909	2.6	52	67	64	83	77	100	86	111	94	122	103	133
3610	2.7	52	69	64	87	77	104	86	115	94	127	103	138
3343	2.8	52	72	64	90	77	108	86	120	94	132	103	143
3104	2.9	52	75	64	93	77	112	86	124	94	136	103	149
2888	3.0	52	77	64	96	77	115	86	128	94	141	103	154
2691	3.1	52	80	64	99	77	119	86	132	94	146	103	159
2510	3.2	52	82	64	103	77	123	86	137	94	150	103	164
2343	3.3	52	85	64	106	77	127	86	141	94	155	103	169
2186	3.4	52	87	64	109	77	131	86	145	94	160	103	174
2037	3.5	52	90	64	112	77	135	86	149	94	164	103	179
1895	3.6	52	92	64	115	77	138	86	154	94	169	103	184
1756	3.7	52	95	64	119	77	142	86	158	94	174	103	189
1615	3.8	52	98	64	122	77	146	86	162	94	178	103	195
1462	3.9	52	100	64	125	77	150	86	166	94	183	103	200
1190	4.0	52	103	64	128	77	154	86	171	94	188	103	205

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

DESIGN FACTORS FOR A DESIGN SPEED OF 60 MPH (URBAN) USING E= 4 % MAX.

					F	PAVEME	NT WID	тн					
RADIUS	E	24	FT	36	FT	48	FT	60	FT	66	FT	72	FT
(FEET)	(%)	D	ESIGN	SOFTW	ARE E	JUIVALE	ENTS (N	NUMBER	R OF L	ANES	AT LAN	IE WIDT	H)
		1 @	12'	1.5	2 12'	2 @	12'	3 @	10'	3 @	2 11'	3 €	12'
		CR	LS	CR	LS	CR	LS	CR	LS	CR	LS	CR	LS
10000	NC	0	0	0	0	0	0	0	0	0	0	0	0
7131	2.0	54	54	67	67	80	80	89	89	98	98	107	107
6663	2.1	54	56	67	70	80	84	89	94	98	103	107	112
6232	2.2	54	59	67	74	80	88	89	98	98	108	107	118
5829	2.3	54	62	67	77	80	92	89	103	98	113	107	123
5451	2.4	54	64	67	80	80	96	89	107	98	118	107	128
5092	2.5	54	67	67	84	80	100	89	112	98	123	107	134
4746	2.6	54	70	67	87	80	104	89	116	98	128	107	139
4408	2.7	54	72	67	90	80	108	89	120	98	132	107	144
4098	2.8	54	75	67	94	80	112	89	125	98	137	107	150
3818	2.9	54	78	67	97	80	116	89	129	98	142	107	155
3563	3.0	54	80	67	100	80	120	89	134	98	147	107	160
3330	3.1	54	83	67	104	80	124	89	138	98	152	107	166
3114	3.2	54	86	67	107	80	128	89	143	98	157	107	171
2913	3.3	54	88	67	110	80	132	89	147	98	162	107	176
2724	3.4	54	91	67	114	80	136	89	152	98	167	107	182
2544	3.5	54	94	67	117	80	140	89	156	98	172	107	187
2372	3.6	54	96	67	120	80	144	89	160	98	176	107	192
2202	3.7	54	99	67	124	80	148	89	165	98	181	107	198
2030	3.8	54	102	67	127	80	152	89	169	98	186	107	203
1842	3.9	54	104	67	130	80	156	89	174	98	191	107	208
1505	4.0	54	107	67	134	80	160	89	178	98	196	107	214

NOTE:

CR AND LS VALUES IN FEET.

FOR PAVEMENT WIDTHS GREATER THAN 72 FEET USE LS VALUES DEVELOPED BY THE DESIGN SOFTWARE.

LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, AND LS VALUES.

TRANSITION CURVES - URBAN

60 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

TO	-5.0	1																																																				
	IPS	FT	ΓS	0 41	43	45	49	51	53	55	ر م	61	63	65	67	71	7.3	75	77	79	10	5 2	87	89	92	94	96	100	100	104	106	108	112	114	116	118	120	124	126	128	130	132	136	138	140	147	146	148	150	152	154	158	160	162
	ANGE RAMPS	Ι α	CR	0 41	41	4	41	41	41	41	4	41	41	41	41	4 4	41	41	41	41	1 1	4	41	41	41	41	14	4 4	4 1	41	41	41	4 41	41	41	41	11	4 1	41	41	41	4	41	41	14	4 1	41	41	41	41	11	4 1	41	41
MAX	INTERCHANGE	FT	LS	0 33	40	42	46	48	20	52	24	28	09	61	63	57	à g	7	73	75	1	2 8	82	84	86	88	8 8	92	4,0	88	100	101	10.5	107	109	111	115	117	119	120	122	124	128	130	152	136	32 2	140	141	143	7 7	149	151	153
8.	INTER	10	CR	39	39	39	39	39	39	39	95 05	39	39	39	39	55 05	65.	39	39	39	20	20 00	39	39	39	39	39	50 0	80 %	39	39	39	90 50	39	39	39	50	80 %	39	39	39	25	39	39	39	202	0 0 0	39	39	39	25	39	39	39
Э П	FT		*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.2	4.4	2.8	3.0	3.2	4.2	3.8	4.0	4.2	4 4	5 u	5.2	5.6	5.8	6.2	0.0	7.6	9.2
USING	WIDTH=48 FT	2 @ 12'	۲S	0 64	52	5.6	29	61	64	99	69	73	92	78	2 2	85 86	2000	8 8	93	95	90	2 2	105	108	110	112	115	120	122	125	127	129	132	137	139	153	157	16.7	169	173	17.7	180	189	193	198	202	212	217	222	227	233	246	254	270
	OIW (I		CR	0	49	4 0 4	04	49	49	49	2 4	49	49	49	49	94 0	64	49	49	49	y 6	2 4	64	64	49	49	64	y 4	2 4	49	49	49	4 4 9 4	49	49	53	54	ب م	55	55	56	56	57	57	82 5	000	200	609	09	61	62	64	65	68
(RURAL)	4 FT	N -	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0 0	0.0	0.0			0.0	0.0	0.0	0.0	0 0	0.0	2.0		7.7	5.7		2.8	2.9	3.1	3.5	3.8	4.6
MPH (LS	33	35	38	39	41	43	44	9 4 6	49	51	52	54	57	59	09	62	62	60	09	2	72	73	75	//	χ Q	000	83	85	98	80 6	91	93	95	96	၀ ၀	101	103	104	106	109	120	122	127	120	132	134	137	140	145	149	155
20 M	-	7	CR	33	33	33	33	33	33	33	55	33	33	33	33	3 5	33	33	33	33	22	3 5	33	33	33	33	33	25 2	25 25	33	33	33	33	33	33	33	55	52	33	33	33	25	33	36	36	36	3,5	37	37	37	20/2	38	38	39
OF 2	FT		м	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2 0	0.0	0.0	0.0	0.0	0.0	0.0	2 0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.1	7.7	2.4	2.5	2.6	2.7	2.9	3.0	3.1	2.5	ر ا ا	3.6	3.8	3.9	1.4	4 4 ن ت	8.4	5.6
	WIDTH=22	1 @ 11	rs	0 02	32	33	38	38	39	41	42	45	47	48	50	51	5 4	55	57	58	00	- 6	5 49	99	67	69	2 5	7.7	2 1,	76	78	79	20 62	84	85	95	97	5	103	105	107	109	113	115	2 €	122	125	127	129	132	135	140	144	150
SPEED	MID	0 2	S	30	30	30	8 8	30	30	30	2 %	30	30	30	30	2 %	8 8	30	30	30	200	2 5	8 8	30	30	30	30	2 2	3 %	30	30	30	30	30	30	33	55	25	34	34	34	24	34	34	35	25	7,7	35	35	36	36	36	37	38
IGN	WIDTH=20 FT WIDTH=22 FT	200	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0	0.0		0.0	0.0	5 0	200	2.0	2.1	2.2	2.2	5.5	5.7	2.5	2.5	2.6	7.7	2.9	2.9	3.0	5.1	۲.۲	3.4	3.5	3.6	γ.,	3.9	4.0	1.4	7.4	. 4	4.6	4.8	4.9	5.1 7.1	5.5	5.8	9.9
DESIGN	WIDTH=20 FT	1 @ 10'	LS	0 88	29	30	33	34	36	37	28	5 1-	42	44	45	46	64	50	52	53	22	200	64	99	89	69	1,1	5/	76	78	80	20	S S	87	89	91	29.2	2 6	66	101	103	105	109	111	113	117	120	122	124	127	129	135	138	144
R			CR	0 82	28	28	28	28	28	28	287	28	28	28	28	287	28	28	28	28	200	07 02	30	30	31	30	31	2 5	2 12	315	31	31	2 5	32	32	32	32	32	32	33	33	2,5	33	33	33	22	25	34	34	34	25	35	35	36
FOR	FT		*	0.0	2.1	2.1	2.2	2.2	2.3	2.3	2.5	2.4	2.5	2.5	2.5	2.6	2.7	2.7	2.8	2.8	6.7	4.9	3.0	3.0	3.1	3.2	3.2	ئ د د	5.5	3.5	3.5	3.6	ر ا ا	3.9	3.9	4.0	4 ·	7.4	4.4	4.5	4.6	γ. α	6.4	5.0	5.1	7.0		5.6	5.8	5.9	6.1	6.5	6.8	7.6
ACTORS	WIDTH= 18	1@ 9-	ΓS	0 09	09	90	3 9	60	09	09	9 6	99	09	09	99	20 00	09	09	09	9	00	20 60	62	63	65	99	200	71	77	75	9/	78	80	83	85	87	68	26	94	96	86	9 5	104	106	108	2 5	717	1	119	122	124	130	133	139
FACT	MID		CR	0 09	58	55	20	48	47	45	45	40	39	38	37	δ 7,	34	33	32	31	200	2 8	29	29	29	29	29	3 8	202	300	30	30	200	2000	30	30	ری در	ار 12	3,15	31	33	2 2	32	32	32	32	30	33	33	33	55	34	34	35
	<u> </u>	-		NC 2.0	2.1	2.2	2.4	2.5	2.6	2.7	7. g	3.0		3.2	3.3	ر 4. د	3.5	3.7	3.8	3.9	O. 4	4. 4 . C	4.3	4.4	4.5	9.4	7.4	φ. 4 Σ	4 ن ح	5.1	5.2	5.3	٠. ۲ ۲. ۲	5.6	5.7	5.8	ა ა. ი	0.0	6.2	6.3	6.4	0.5 C.5	6.7	8.9	6.9	7.1	7.2	7.3	7.4	7.5	7.6	7.8	7.9	8.0
DESIGN	DESIGN	=20	RADIUS(FT)	1800	1148	1036	987	941	899	860	824	759	729	701	674	626	604	582	562	543	524	489	473	457	442	427	413	385	372	358	345	332	308	297	286	276	266	248	240	232	225	21/	202	196	189	176	170	164	158	152	146	132	124	108

TRANSITION CURVES - RURAL
20 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

w VALUES.

w VALUES IN FEET. LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, LS, AND

	FT	LS	0	43	ξ α	50	52	54	56	80	63	65	67	69	7,5	75	78	80	84	86	88	90	95	97	99	10.1	105	108	112	114	116	130	123	125	120	131	133	135	140	142	144	148	150	153	155	159	161	16.5	168	170	172
WIDTH	1 00	CR	0	43	7 4	43	43	43	2 4 5	5.4	43	43	43	43	2 4 5	43	43	43	54	43	43	43	5 4	43	43	0 4	43	43	43	43	43	4 ک 4 ک	43	43	4 م م	43	43	43	43	43	4 5	43	43	43	43	43	43	24 5	43	43	43
2	FT	LS	0	40	7 5	46	48	20	52	1 95	58	09	62	64	99 89	2 2	72	74	782	80	82	84	0 8	06	92	94	98	100	102	106	108	13	114	116	2138	122	124	126	130	132	134	138	140	142	144	148	150	15.4	156	158	160
	16 F	S	0	0 4	5 4	40	40	40	040	40	40	40	40	40	04 04	40	40	040	04	40	40	0 4	0 4	40	40	0 4	40	40	040	40	40	0 4 4	04	40	0 4	04	40	0 4	0 4 4	40	040	404	H	Н	040	04	040	04 04	40	40	40
		*	0.0	0.0		+	\dashv		+	0.0	-	0.0	Н		0.0	+		0.0	+		-	0.0	0.0	0.0	0.0	0.0	0.0		0.0	\vdash	Н	_	+		+	+		+	-	0.0	_	+	\sqcup	Н	_	+	Н	5.6	4.2	Н	
	12.	1,0	H	52 (+) (2	+	+		Н	-	+	+		_	101			108	14		119	121			132 (137 (Н	142	-	150 (+	+		+		170	_	+	+	H	-	+	+	+	236	Н	
Î	7			52	+	7 7	7	2	200	10	1 0	20	2	2	2 5	1 2	2 6	20 0	7 2	1 10	1(200	7 0	2 2	200	7 0	2 1	1.	2 2	1,	1.	52 1	+	+	+	52 1	H	+	52 1	H	9 1	_		<u></u>	+	59 2	+	60 2	+	62 2	
■ MIDTH)		+	Н	0.0	+		0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0 0	0 0	0	0 0	0 0	0 5	0	0 0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0	0.0	_	+	\dashv	+	+		+	-	\vdash		+	0.0	Н	+	0.0	Н	0.0	+	2.3	_
LANES AT LANE	12.	*		+		+	\dashv	_	0.0	+	+	\vdash			0.0	+		+	0.0		0.0	0.0	j c + ′′		_		+	_	+	\vdash	\vdash	+	+	+	+	0.0		\dashv	0.0	\vdash		+	+	Н	-	+	6	+	+	Н	_
VES A	100	Ë	0	1	200	1	-	_	45	+	+	\vdash			5/2	+		+	67	69	.2	7	7,	78	79	Σ α	84		88	-	Н	95		+	+	105	Н	+	112	\vdash	115	+	Н	Н	+	127	12	131	+	4	+
S S S		R		35	+	2 0	3		35		+	-			35	1		,,,,	35	35	35	35	3,5	35	35	3 5	35	35	35	35	3	35) W	3	35	+		-	35	\vdash	35	35	35	Н	+	35	100	35	+	38	
		*	0.0	0.0	2 0	0.0	0.0	0.0	0.0	9 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0	0 0	0.0	0.0	0 0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	2.0	2.2	2.2	2.3	2.4	2.6	2.	-	2 6	3.3	ω.
	10 11	rs	0	32	5 5	37	38	40	41	4 4	46	4	49	51	52	55	57	59	62	63	9	99	8 2	7 1	73	76	77	79	81	84	85	γ α	8 06	92	93	96	86	66	103	104	115	120	121	124	126	131	133	135	140	143	148
/ EZ		S	0	32	20 02	33	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	32	35	35	35	35	35	36	36	36	36	37	37
SOF I WARE EQUIVALENTS INUMBER		*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.0	2.1	2.2	2.2	2.4	2.4	2.5	2.6	2.7	2.7	2.8	2.9	3.0	3.2	3.2	3.3	3.4	3.6	3.7	3.8	4.1	4.3	4.8
NAKE	1 @ 10		0	29	3 5	33	35	36	38	40	42	43	45	46	848	202	52	53	26	58	59	9	79	65	99	200	200	80	23	84	98	88 8	92	93	95	66	101	103	105	108	11 11	115	116	119	121	125	127	130	135	138	142
		S	0	29	200	29	29	59	29	200	29	29	29	29	29	29	29	29	29	29	29	29	67	29	29	67 00	29	32	32	32	32	32	33	33	33	33	33	33	33	33	34	34	34	34	34	4 45	34	35	35	35	36
DESIGN		>	0.0		0.0	2.0	2.1	2.1	2.1	- 0 0	2.2	2.2	2.3	2.3	2.3	2.4	2.4	2.5	2.0	2.6	5.6	2.7	7.7	2.8	8.8	20.00	2.9	3.0	3.0	3.1	3.2	3.2	3.4	3.4	3.5	3.6	3.7	3.7	2 0	3.9	0.4	4.2	4.2	4.3		7 4	4.7	4 ر 8 ر	5.1	5.3	2.8
_	9	LS.	Н	26	+	1	80	80	080	2 6	+	8	Н	+	08 68	+	\vdash	80	8 8	80	80	88		8 8	08		80	80	08 08	88	82	48	88	68	50 6	95	97	86	100	104	+	110	+	Н	116	+	+	124	+	132	-
	-	SR	0	26	/ 2/	2 2	\dashv	+	60	2 8	26	54	52	20	0 4 4 0 0 4	46	45	44	24	40	40	39	37	36	35	0 4 5 4 4 5	33	32	32	31	31	712	31	31	12 12	32	32	32	32	32	32	32	32	33	33	333	33	55	34	34	34
		(%)		2.0	1.7	1	2.4			_		_	Ш	_	5.5 4 4	╄		3.7	_		-	4.2	4 4 υ 4	4.5	4.6	4 4 ν α	4.9	5.0	5.7	5.3	4.0	ر د ر د	5.7	5.8	D. C.	0.10	5.2	5.3	5.5	9.9		_	Н	Ш	4	\perp	7.5	9.7	7.8	7.9	0.0
VELUCII T	-25	1		+	+	t	H	1	\dagger		+	\vdash	Н		+	+	Н	+	+		\dashv						+			t								\dagger		H			H	H		+	239	+	T	H	
	CATI																	ΤF							V											7	4[_													
																			2	25	Ö	\setminus	ΛF	7	+	D	Ε:	SI	GI	\bigvee		SF) E	E	D																

T	C-5.0	1																																																_	_	_	_
	PS	FT	LS	0 4	48	50	52	57	59	61	63	900	00/2	72	75	79	81	84	98	8 6	93	95	97	99	104	106	108	=======================================	5 5	117	120	122	124	129	131	133	138	140	142	147	149	151	153	156	160	162	165	167	169	174	176	180	00
	ANGE RAMPS	∞	CR	0 4	45	45	45	ر بر در	45	45	45	4 د	5 4 5	45	45	45	45	45	45	0 4 7 4 7	45	45	45	დ /	45	45	45	45	4 7 ر	45	45	45	45	45	45	45	45	45	45	5 4 5	45	45	45	45	545	45	45	45	4 ک 7 م	45	45	45	4°
8% MAX.	INTERCHANGE	F	LS	0 4	45	47	200	22	56	28	09	7.9	67	69	77	75	77	79	82	40 8	88	90	92	94	66	5 5	103	105	100	3 =	114	116	2 18	122	124	126	131	133	135	139	141	143	146	148	152	154	156	158	160	165	167	169	-
%%	INTER	16	CR	٥٢	43	43	43	ر ا ا ا	43	43	43	24 6	0 4 5	43	43	5 4 5	43	43	43	0 4 5	43	43	43	4 د د د	5 4 5 4	43	43	43	5 4 5 6	43	43	43	43	43	43	43	5 4 5	43	43	54 45	43	43	43	43	5 4 5	43	43	43	4 5 4 5	43	43	43	4°C
<u>"</u>	F		3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	2.0	2.7	2.6	2.8	0.5	0.0
SING	WIDTH=48 F	0 12	LS			09	+	+		74	+	+	85		90				+) 	+		118		126			+	140	-			+	156	Н	+	167		172		+	Н		189	+	+	H	219	224 228	233	+	243	_
	WIDT!	2	CR		25		+	22 22			+	+	55		55	+	\vdash	\vdash	+	555	\vdash	22	+	Ť	55	+		55	7.7	+	Н	55 1	55 1	+	H	+	55	55 1	55 1	Ŧ.	+	Н	+	55	+	+	Н	+	+	61	+	62 2	-
(RURAL) USING	WIDTH=20 FT WIDTH=22 FT WIDTH=24 FT WIDTHSOFTWARE FOLIVALENTS (MIMBER OF LANES AT LANE WIDTH)		*		0.0		0 0				0,0	0 0			0.0	+	0				+	0.	0	+	0.0	\vdash	0.	0,0		+	0	0,	0 0		0	+		0				Н	0.	0.0			0	0	0 0	0 0	\vdash	0.0	4
	24 FT	@ 12'	\vdash		39 0	Н	42 0.	+	+-		51 0	+	0 0			+	0 99		70	73		7 0	79 0	+	+		88 0.	+	+	+		\dashv	0 0	104 0.	Н	+	+	3 0	+	5 0	1	Н	124 0	126 0	30	+	Н	135 0.	0 2	0 0	+	144 0	\dashv
MPH	WIDTH=24	10	S LS			4	4	4 4	4		5 .	V V	0 7	5	09 2	64	9 /	9 /	7		7 7	7	7	ρα	84	2 0	2	6	9 2	6	7 97	6	2 2	2 2	7	2 5	11	11	1 1	119	12	7 12	12	7 12	12 2	15	7	13) t	7 4	++	+	
30	OF -	5	CR	0 0	+	37	3,	3 6	3,5	37	3,	ς _κ	0 6	3/3	3,7) (v)	3,7	37	3,	0 6	37	37	3,	ر د د) (S	3,7	37	3,5	5 6	3,0	37	3,	3,	3 6	37	3,	3 6	37	3,) k	3/5	37	3,	3,	5 12	3/5	37	3,	3 2	3/5	\vdash	37	_
OF.	2 FT	1	3		0.0	0.0	+	0.0	+		0 0	+	0.0		0.0	+	0.0		+	0.0		o.		0.0	0.0	0.0	o.	+	0 0	+	Н	0 0	+	0		+	0.0	0.0	0.0		+	Н	0.0		0.0	+	o.	2.0	7 0	2.7	+	2.5	-
SPEED	WIDTH=22	1 @ 11	LS		35	37	39	4 0	44	45	47	4 G	52	54	55	5 2	09	62	64	67	69	70	72	75	77	79	80	82	0 7	87	88	06	92	95	97	99	102	104	105	109	12	112	114	115	119	120	122	135	15/	142	1	147	
	W FNT		S	0 2	34	34	34	2 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	34	34	34	2 2	4 5	34	34	34	34	34	34	4 45	34	34	34	24	34	34	34	34	4 4	34	34	34	34	34	34	34	34	34	34	4 5	34	34	34	34	34	34	34	37	2,7	37	38	38	၁
DESIGN	FT	7	*	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0) C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0)))	0.0	0.0	0.0	2.0	2.0	2.1	2.1	2.2	2.3	2.3	2.4	2.5	2.5	2.6	2.6	7.7	2.8	2.9	3.0	ر د د د	3.3	3.4	3.5	٥. ت
	WIDTH=20	1 0 10	۲S	0 2	32	34	35	ري عه	40	41	43	4 4	47	49	50	54	55	22	28	61	63	64	99	/9	02	72	73	75	2 82	6/	81	82	9 5	8 6	100	001	103	105	107	111	113	115	117	119	123	125	127	129	132	136	139	141	∓ €
A A			CR	0	31	31	31	2 5	31	31	31	15		31	31	2 5	31	31	31	ر ا	31	31	31	2 5	2 5	31	31	31	2 12	31	31	31	37	36	35	34	34	34	34	35	35	35	35	35	35	35	35	35	35	36	36	36	'n
FOR	FT PESIGN		>	0.0	0.0	0.0	0.0	0.7	2.0	2.1	2.1	2.1	7.7	2.2	2.2	2.3	2.3	2.3	2.4	7.4	2.5	2.5	2.5	0.7	2.6	2.7	2.7	2.7	2 00	2.8	2.9	2.9	0.0	3.0	3.1	3.1	3.2	3.3	3.3	4.4	3.5	3.5	3.6	3.6	3.8	3.8	3.9	4.0	4	4.3	4.4	4.5	τ. ⁺
FACTORS	2	6 0	LS	0 8	29	30	32	3 6	100	100	100	2 5	3 2	100	9 5	3 2	100	100	9 5	3 6	100	100	00 5	3 2	3 2	9	100	20 5	3 5	001	100	100	9 5	3 2	100	9 5	3 2	101	102	106	108	110	112	113	2 2	139	122	124	126	131	133	135	501
ACT	#HTQIM		C.R.	0 0	28	28	788	α 4 α	77	75	72	69	65	63	19	58	56	55	53	50	49	48	47	4 4 5 4 5	2 4 4	43	42	14 5	5 4	39	38	38	37	36	35	34	33	33	33	33	33	33	33	33	34	34	34	34	24	35	35	35	22
			E(%)	J C	2.1	2.2	2.3	4.7 7.7	2.6	2.7	2.8	2.3	3.1	3.2	3.3	3.5	3.6	3.7	% N	y 4	4.1	4.2		4 .4 7 .4	9.4	4.7	8.8	6.4	2 5	5.2	5.3	5.4	2.5	5.7	5.8	5.9	6.1	6.2	6.3	+ 5	6.6	6.7	8.9	6.9	2.7	7.2	7.3	7.4	7.5	7.7	7.8	6.7).
DESIGN	DESIGN	=30	RADIUS(FT) E		2276		+				1643	\dagger			1352					1061			964		877			800	757	t			663				548		515					427	\dagger				348	320		287	

TRANSITION CURVES - RURAL 30 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

802.36

SPECIFICATION REFERENCE

w VALUES IN FEET. LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, LS, AND w VALUES.

	RAMPS	1	rs.	0 5	51	53	29	28	63	65	0 2	72	75) 8 	82	84	20 00	92	94	96	101	104	901	5 =	113	911	120	123	125	130	135	137	140	144	147	152	154	159	161	166	168	173	176	8/1/8	183	185	190	192	TC-5.0
		∞		0 5	8 4 8	8 4 8	48	8 4 8	48	48	4 8	48	48	6 4 8	48	48	8 4 8	84	48	φ α	48	48	8 0	5 4 8	48	8 0	φ 4 8 4 8	48	84 8	48	84 84	48	48	48	8 4	48	8 4 8	48	8 4 8	48	48	4 8 4	48	8 0	48	48	48	84	UES.
	INTERCHANGE	FT	LS	0 5	48	50	52	55	59	61	66	89	70	75	77	79	82	98	88	91	95	97	100	104	106	109	113	115	118	122	127	129	131	136	138	142	145	149	151	156	158	163	165	167	172	174	176	18	w VALUES
MAX.	INTER	16	CR.	0 ;	46	46	46	46	46	94	4 6	46	46	4 0 4	46	46	4 4 6	46	46	9 4	46	46	46	46	46	46	4 4	46	46	46	4 4 6	46	46	46	46	46	9 4 6	46	4 4	46	46	46	46	46	40	46	46	46	AND w
8%	FT	_	3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.1	2.4	3.0	3.9	LS,
= =	WIDTH=72	0 12	LS	0 5	82	86	06	93	101	105	113	117	120	128	132	136	140	148	151	155	163	167	171	179	182	186	194	198	202	210	213	221	225	233	237	244	248	256	260	268	271	279	283	287	312	318	325	344	; E, CR,
	WID	K	, R	0 5	78	78	78	78	78	78	0 8	78	78	0 8/	78	78	20 00	78	78	28 x	78	78	8 %	2 82	78	8 6	0/8/	78	200	78	78	78	78	78	78	78	78	18	8/8/	78	78	0 82	78	20/00	83	83	84	98	NDING
NSING	L 5	r -	>	0.0	0.0	0.0	0:0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.6	CORRESPONDING
(JK)	WIDTH=48	2 @ 12	1,0	0	59	64	67	70	92	79	85	88	90	96	66	102	103	=	114	117	122	125	128	134	137	140	146	149	151	157	163	166	169	175	178	183	186	192	195	201	204	210	212	275	221	224	227	258	
(RUR,	WIDT	LANE	CR	0	59	59	59	59	59	29	200	59	59	50	59	59	59	59	59	50	59	59	59	59	59	59	59	59	59	59	59	59	59	59	59	59	59	29	59	59	59	59	59	59	59	59	59	65	R THE
	FT		>	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	JS FOR
MPH	H=24	LANES @ 12"	101	0 5	41	243	45	47	51	53	57	59	09	64	99	68	72	74	9/	78	82	84	98	8 6	16	93	97	66	103	105	109	11	113	117	119	122	124	128	130	134	136	140	142	144	148	150	151	155	RADIUS
35	WIDTH=	۲ 5	CR	0 5	39	39	39	33	39	39	39	39	39	39	39	39	39	39	39	39	39	39	39	39	39	59	39	39	39	39	39	39	39	39	39	39	39	33	39	39	39	39	39	20	39	39	39	39	/ABLE
OF	FT W	NOMBE NOMBE	>	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.3	ALLOWABLE
SPEED			rs I		38	0 4 0	14	43	47	84 8	22	54	55	59	61	63	64	89	70	71	75	77	67	82	84	986	89	16	93	96	200	102	103	107	901	112	114	138	171	123	125	128	130	132	135	137	15.3	157	MINIMUM
	WIDTH=	EQUIVALENTS	SR.	0 5	36 36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	40	
SIGN	- L	7 L	3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.0	2.1	2.1	2.2	2.2	2.3	2.4	2.5	2.5	2.6	2.7	2.8	2.9	3.3	IS THE
DE	I١	- W AR		+	34	36	38	39	42	44	474	49	50	54	55	57	60	62	63	65	68	70	77	75	76	8/8	81	83	86	88	91	92	94	120	120	120	120	120	120	125	127	131	133	135	140	142	145	151	RADIUS
\ \ \ \	WIDTH=20	7 00	CR	0 1	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	33	55	33	33	33	33	33	33	33	40	40	39	38	37	36	37	37	37	37	27	37	37	38	388	
FOR	FT	DE SIGN	3		0.0	0.0	0.0	0.0	2.0	0.0	0.10	2.1	2.1	7.2	2.2	2.2	2.2	2.3	2.3	2.3	4.4	4.2	4.0	2.5	2.5	9.0	0.2	5.6	2.7	2.7	00 00	8.9	6.0	3.0	3.0	3.1	3.1	3.2	5.2	3.3	4.6	4.0	3.5	5.6	3.7	3.8	0.7	5.4	. LISTED
)RS	H= 18	- 0	1 (/)		31 (32 (34	35	120	20	2 2	120	20	20	20	120	20	20	120	20	20	20	20	20 20	120	20	20	120	20	20	20 20	120	20	120	20	20	20	202	20	20	121	25	127	23	33	136	141	44	FEET.
ACTOR	#HLQIM	1	CR	+	30 %	30	30	30	93	89	83	80	78	73	71	69	65	64	62 1	909	58	26	55	53	52	20	2 4 8 4 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	48	47	45	44	43	42	40	40	39 1	38	37	36	35 1	35	35	35	35	35	36	36	36	IES IN
			12	20		2.2	2.3	2.4	5.6	7.7	0.7 6.	3.0	2.1	3.3	3.4	3.5	5.6	3.8	3.9	0	- 5	4.3	4 4	5 4.	4.7	ω	5.0	5.1	5.3	4.0	0.0	2.7	8.0	3.0	6.1	5.3	4.0	9.9	/ 00	5.9	7.0	7.2	7.3	۲.4	7.6	7.7	8.7		VALUES
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VIRGINIA DEPARTMENT OF TRANSPORTATION

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DESIGN	-			≥ 0:0	0.0	0 0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	2.0	2.0	2.1	2.1	2.2	2.3	2.4	2.4	2.5	2.6	2.7
0 V	WIDTH=20		10 10	0 [2	35	37	4 0	42	44	47	47	50 49	52	54	57	59	67	64	99	88	7 69	73	75	78	8	83	85	88	8	94	95	66	100	102	106	107	3 =	124	128	130	132	136	139	143	145	148	152	155
FOR	3	S NOIS	ç	50	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	39	39	39	39	39	39	39	39	39	39	40
	18 FT	DES		» O.O	0.0		0.0			0.0		2.0	2.0	2.1	2.1	2.1	2.2	2.2	2.2	2.2	2.5	2.3	2.3	2.4	2.4	2.4	2.5	2.5	2.6	2.6	2.7	2.7	2.7	2.8	2.8	2.9	2.9	3.0	3.0	3.1	3.1	3.2	3.3	3.4	3.4	3.5	3.6	3.7
ACTORS	WIDTH= 1		9 -	20	32	33	36	38	39	42	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	122	124	126 128	130	133	137	139	141	146	148
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		>	:	NC C.	2.0	2.1	2.3	2.4	2.5	2.7	2.7	2.9	3.0	3.1	3.3	3.4	3.5	3.7	3.8	3.9	0.4 U.4	4.2	4.3	4.5	4.6	4.7	4.9	5.1	5.2	5.4	5.5	5.7	5.8	5.9	6.1	6.2	6.4	6.5	6.7	8.9	6.9	7.1	7.2	4.7	7.5	7.6	7.8	7.9
DESIGN	DESIGN	VELOCITY =40	- ⊢	_		\top	3430				П			2443	2269	2190	2115	1977	1913		17.39	1686	1587	1540	1495	1452	1370		1258	1188	1154	1090		1028		944			818		771		703	658	635	612	560	529

TRANSITION CURVES - RURAL 40 MPH DESIGN SPEED VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

No. Color State Color	SPE(VE SIGN	-	֝֟֟֝֟֝֟֝֟֝֟֝֟ ֞֞֓֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞֞	WIDTH= 18	18 FT ×		WIDTH=20	7 L	5	WIDTH=22 F			WIDTH=24 FT	- 1	WIDTH=48	WIDTH=48	F1		WIDTH=72 FT		MAN.	INTERCHANGE	RAMPS	PS
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Mary		4702		34	35	0.0	38	39	0.0	1 4	43	0.0	45	47	0.0	67	200	0.0	68	94	0.0	53	+	+	59
No. 1,		4467	2.2	34	37	0.0	38	14 ;	0.0	14 1	45	0.0	45	49	0.0	67	74	0.0	68	98	0.0	53	-		61
		4724	2.7 4.0	45	80	0.0	00 00	45		4 4	4 04	2 0	45	54	2 0	67	\ &		200	10.5		5,7	+.	+	ی ام
		3876	2.5	34	42	0.0	38	47	0.0	14	51	0.0	45	56	0.0	67	84	0.0	83	112	0.0	53	99	99	
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,		3710	2.6	34	44	0.0	38	49	0.0	41	53	0.0	45	58	0.0	29	87	0.0	88	116	0.0	53	69	99	72
		3554	2.7	34	45	0.0	38	200	0 0	4 2	55	0.0	45	9 5	0.0	67	06	0.0	68	120	0.0	53	7 ;	9 9	
Name		3412	2.8	45	4	0.0	200	52	0.0	4 4	200	0.0	4 to	50	0.0	/9	46	0.0	S 0	125	0.0	52	74	و ا	٥ ۲
1969 11, 11 11 11 11 11 11 11		3152	3.0	34	50	0.0	388	56	0.0	4	62	0.0	45	67	0.0	67	90	0.0	68	134	0.0	53	, 62	2 9	2 8
Table Tabl		3035	3.1	34	52	0.0	38	58	0.0	41	64	0.0	45	69	0.0	67	104	0.0	68	138	0.0	53	82	99	86
2866 3.3 3.4 5.6 0.0 4.1 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 8.1 1.0 0.0 9.1 9.0 0.0 9.0 <td></td> <td>2925</td> <td>3.2</td> <td>34</td> <td>54</td> <td>0.0</td> <td>38</td> <td>09</td> <td>0.0</td> <td>41</td> <td>99</td> <td>0.0</td> <td>45</td> <td>72</td> <td>0.0</td> <td>67</td> <td>107</td> <td>0.0</td> <td>89</td> <td>143</td> <td>0.0</td> <td>53</td> <td>84</td> <td>92</td> <td>89</td>		2925	3.2	34	54	0.0	38	09	0.0	41	99	0.0	45	72	0.0	67	107	0.0	89	143	0.0	53	84	92	89
282.2 3.3 8.4 9.2 4.4 9.0 </td <td></td> <td>2866</td> <td>3.3</td> <td>34</td> <td>55</td> <td>0.0</td> <td>38</td> <td>62</td> <td>0.0</td> <td>14</td> <td>89</td> <td>0.0</td> <td>45</td> <td>74</td> <td>0.0</td> <td>67</td> <td>19</td> <td>0.0</td> <td>83</td> <td>147</td> <td>0.0</td> <td>53</td> <td>87</td> <td>90</td> <td>92</td>		2866	3.3	34	55	0.0	38	62	0.0	14	89	0.0	45	74	0.0	67	19	0.0	83	147	0.0	53	87	90	92
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		2631	٠. ٢	8	140	2.1	0 00	65		4 4	2 2	0.0	45	2 8	2 0	20	117		20 00	+	2.0	53	2 6	+	والإ
2.451 3.3 7.6 1.0 2.4 8.3 0.0 4.5 8.3 0.0 6.7 2.4 1.0 0.0 8.3 0.0 6.7 2.4 0.0 8.3 0.0 8.3 0.0 8.3 0.0 8.3 0.0 9.3 0.0 8.3 0.0 9.3 0.0 </td <td></td> <td>2544</td> <td>3.6</td> <td>78</td> <td>140</td> <td>2.1</td> <td>38</td> <td>67</td> <td>0.0</td> <td>4</td> <td>74</td> <td>0.0</td> <td>45</td> <td>80</td> <td>0.0</td> <td>67</td> <td>120</td> <td>0.0</td> <td>68</td> <td>+</td> <td>0.0</td> <td>53</td> <td>35</td> <td>+</td> <td>3 3</td>		2544	3.6	78	140	2.1	38	67	0.0	4	74	0.0	45	80	0.0	67	120	0.0	68	+	0.0	53	35	+	3 3
2.9.8.3 2.9.8.4 <t< td=""><td></td><td>2461</td><td>3.7</td><td>9/</td><td>140</td><td>2.2</td><td>38</td><td>69</td><td>0.0</td><td>41</td><td>9/</td><td>0.0</td><td>45</td><td>83</td><td>0.0</td><td>67</td><td>124</td><td>0.0</td><td>83</td><td>+</td><td>0.0</td><td>53</td><td></td><td>H</td><td>100</td></t<>		2461	3.7	9/	140	2.2	38	69	0.0	41	9/	0.0	45	83	0.0	67	124	0.0	83	+	0.0	53		H	100
23.30 4.0 7.0 4.0 7.0 8.0 1.4 0.0 8.0 1.4 0.0 8.0 1.0 4.0 8.0 9.0 1.0 4.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 </td <td></td> <td>2383</td> <td>3.8</td> <td>74</td> <td>140</td> <td>2.2</td> <td>38</td> <td>71</td> <td>0.0</td> <td>41</td> <td>78</td> <td>0.0</td> <td>45</td> <td>85</td> <td>0.0</td> <td>29</td> <td>127</td> <td>0.0</td> <td>68</td> <td>169</td> <td>0.0</td> <td>53</td> <td>001</td> <td>99</td> <td>106</td>		2383	3.8	74	140	2.2	38	71	0.0	41	78	0.0	45	85	0.0	29	127	0.0	68	169	0.0	53	001	99	106
2.65		2308	3.9	72	140	2.2	38	73	0.0	4	80	0.0	45	87	0.0	67	130	0.0	83	174	0.0	53	103	92	108
2.004 4.1 6.		2237	0.4	2	140	2.2	38	75	0.0	4	82	0.0	45	68	0.0	67	134	0.0	68	178	0.0	53	105	9 9	: ≓
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1962 4.4 64 40 2.3 38 62 0.0 41 90 0.0 45 90 0.0 67 41 90 0.0 90 90 90 90 90 9		204	7.4	6	140	د.ک د د	000	0 0		4 4	8 8	2.0	4 2 4 2	94	2 0	67	144	0 0	0 0	107		53	13	0 6	= 5
1924 4.5 6.3 14.0 2.4 3.8 6.4 0.0 4.1 9.2 0.0 4.5 10.0 0.0 6.7 15.0 0.0 6.9 2.00 0.0 0.5 3. 12.1 1870 4.5 6.1 14.0 2.4 3.8 6.8 0.0 4.1 9.6 0.0 4.5 10.0 0.0 6.7 15.4 0.0 0.0 6.7 15.4 0.0 0.0 5.3 12.1 1870 4.7 6.0 14.0 2.4 3.8 8.9 0.0 4.1 9.6 0.0 4.5 10.0 0.0 6.7 15.4 0.0 8.9 2.0 0.0 5.3 12.1 1870 4.5 6.8 14.0 2.4 3.8 8.9 0.0 4.1 9.6 0.0 4.5 10.0 0.0 6.7 16.0 0.0 8.9 2.2 0.0 5.3 12.1 1864 5.5 5.6 14.0 2.4 3.8 8.9 0.0 4.1 10.0 0.0 4.5 10.0 0.0 6.7 17.4 0.0 8.9 2.2 0.0 5.3 13.4 1855 5.4 14.0 2.5 3.8 9.9 0.0 4.1 10.0 0.0 4.5 11.8 0.0 6.7 17.4 0.0 8.9 2.2 0.0 5.3 13.4 1856 5.4 5.1 4.0 2.5 3.8 9.9 0.0 4.1 10.0 0.0 4.5 11.8 0.0 6.7 17.4 0.0 8.9 2.2 0.0 5.3 14.5 1857 5.5 5.0 4.0 2.5 3.8 10.0 0.0 4.1 11.8 0.0 4.5 12.0 0.0 6.7 18.4 0.0 8.9 2.2 0.0 5.3 14.5 1858 5.4 5.1 4.0 2.5 3.8 10.0 0.0 4.1 11.8 0.0 4.5 12.0 0.0 6.7 18.4 0.0 8.9 2.4 0.0 5.3 14.5 1859 5.4 6.1 6.1 6.2 3.8 10.0 0.0 4.1 11.8 0.0 4.5 12.0 0.0 6.7 18.4 0.0 8.9 2.4 0.0 5.3 14.5 1850 5.5 6.1 4.0 2.6 3.8 10.0 0.0 4.1 11.5 0.0 4.5 12.0 0.0 6.7 18.4 0.0 8.9 2.4 0.0 5.3 14.5 1850 5.5 6.1 4.0 2.6 3.8 10.0 0.0 4.1 11.5 0.0 4.5 12.0 0.0 6.7 18.4 0.0 8.9 2.4 0.0 5.3 14.5 1850 5.5 6.1 4.0 1.0 2.8 3.8 11.5 0.0 4.1 12.5 0.0 6.7 12.0 0.0 8.9 2.2 0.0 5.3 14.5 1850 6.1 4.1 4.1 0.2 3.8 11.5 0.0 4.1 11.5 0.0 4.5 13.4 0.0 6.7 2.4 0.0 8.9 2.2 0.0 5.3 14.5 1850 6.2 4.4 4.0 2.8 3.1 3.0 0.0 4.1 3.1 0.0 6.7 2.4 0.0 8.9		1982	4.4	64	140	2.3	38	82	0.0	41	8 6	0.0	45	86	0.0	67	147	0.0	68	196	0.0	53	116	99	1 2
4,7 61 140 2,4 38 86 0.0 41 94 0.0 45 10.4 10.0 67 154 0.0 89 20 0.0 51 12.4 0.0 89 10.0 2.4 10.0 89 10.0 2.4 10.0 10.0 4.0 10.0		1924	4.5	63	140	2.3	38	84	0.0	41	92	0.0	45	100	0.0	67	150	0.0	89	200	0.0	53	119	. 99	12
National Column National C		1870	4.6	61	140	2.4	38	98	0.0	41	94	0.0	45	103	0.0	67	154	0.0	89	205	0.0	53	121	. 99	2
170 170		1817	7.	9	140	2.4	88 5	88 8	0.0	- 4	96	0.0	4 1,	205	0.0	2 6	757	0 0	20 00	209	0.0	55	24	9 ,	기약
15.66 5.0 5.0 14.0 2.4 38 95 0.0 41 102 0.0 45 112 0.0 67 107 0.0 89 223 0.0 53 134 15.9		1717	0.4	2 C	140	4.7	0 %	9 6		4 4	9 5	000	7 4	100	2 0	67	164	0.0	200	7 2 4	2 0	5,7	07 06	0 6	<u>يا د</u>
1624 51 56 140 25 38 95 0.0 41 104 0.0 45 114 0.0 67 170 0.0 89 227 0.0 51 134 0.0 89 227 0.0 41 106 0.0 45 114 0.0 67 174 0.0 89 224 0.0 51 134 1536 5.2 51 140 2.6 50 140 2.6 38 100 0.0 41 100 0.0 45 120 0.0 67 180 0.0 67 180 0.0 67 180 0.0 67 180 0.0 67 180 0.0 67 180 0.0 67 180 0.0 41 110 0.0 45 120 0.0 67 180 0.0 41 110 0.0 67 180 0.0 67 120 0.0 67		1669	5.0	56	140	2.4	38	93	0.0	41	102	0.0	45	112	0.0	67	167	0.0	83	223	0.0	53	132	99	13
1579 5.2 5.4 440 2.5 38 9.7 0.0 4.1 106 0.0 4.5 116 0.0 6.7 1.74 0.0 89 2.32 0.0 5.3 140 1405 140 1405 140 140 150 140 1		1624	5.1	55	140	2.5	38	95	0.0	41	104	0.0	45	114	0.0	29	170	0.0	89	227	0.0	53	134	. 99	4
1536 5.4 5.5 140 2.5 38 99 0.0 41 108 0.0 45 108 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 67 118 0.0 65 118		1579	5.2	54	140	2.5	38	97	0.0	41	106	0.0	45	116	0.0	67	174	0.0	83	232	0.0	53	137	,	4
1495 5.5 5.1 140 2.5 38 100 0.0 41 110 0.0 45 125 0.0 67 184 0.0 89 246 0.0 53 145 1415 5.6 5.6 5.0 140 2.6 38 104 0.0 41 115 0.0 45 125 0.0 67 184 0.0 89 246 0.0 53 145 1415 5.6 5.6 5.0 140 2.6 38 104 0.0 41 115 0.0 45 125 0.0 67 184 0.0 89 246 0.0 53 145 130 130 5.8 49 140 2.6 38 106 0.0 41 117 0.0 45 125 0.0 67 194 0.0 89 246 0.0 53 145 150 130 5.8 49 140 2.6 38 106 0.0 41 117 0.0 45 125 0.0 67 194 0.0 89 246 0.0 53 150 130		1536	5.3	53	140	2.5	38	66	0.0	4	108	0.0	45	138	0.0	67	177	0.0	68	236	0.0	53	40	90	4
1415 5.6 5.0 140 2.6 38 104 0.0 41 115 0.0 45 125 0.0 67 187 0.0 89 249 0.0 53 147 137 138 138 104 0.0 41 115 0.0 45 125 0.0 67 187 0.0 89 249 0.0 53 147 137 138 138 104 0.0 41 117 0.0 45 125 0.0 67 187 0.0 89 249 0.0 53 153 147 130 2.5 38 108 0.0 41 117 0.0 45 125 0.0 67 190 0.0 89 249 0.0 53 153 147 130 2.5 38 108 0.0 41 123 0.0 45 134 0.0 67 200 0.0 89 258 0.0 53 153 153 135 1		1495	5.4	52	140	2.5	38	9	0.0	4	13	0.0	45	120	0.0	67	180	0.0	68	240	0.0	53	42	9 .	ŌΓ
1776 57.5 50 140 2.6 38 106 0.0 41 117 0.0 45 127 0.0 67 190 0.0 69 254 0.0 53 150		14.15	0.0 7.0	25	740	0.7	00 00	104		4 4	5 5	2 0	40	125	0.00	67	104	0 0	0 0	240	2 0	57	0 7 7	0 4	ŨĮξ
1339 5.8 49 140 2.6 38 108 0.0 41 19 0.0 45 129 0.0 67 194 0.0 89 258 0.0 53 155 1302 5.9 48 140 2.7 38 110 0.0 41 121 0.0 45 132 0.0 67 20 0.0 89 256 0.0 53 155 1236 6.0 46 140 2.7 38 112 0.0 41 123 0.0 67 204 0.0 89 256 0.0 53 165 1199 6.0 41 120 0.0 41 120 0.0 49 10 0.0 67 20 67 20 0.0 83 150 0.0 41 120 0.0 41 120 0.0 41 120 0.0 45 140 0.0 89 250		1376	5.7	50	140	2.0	385	106	0.0	4	11	0.0	4.5	127	0.0	67	190	0.0	0.00	254	0.0	53	50	2 9	3 5
5.9 4.8 14.0 2.7 38 110 0.0 41 12.1 0.0 45 132 0.0 67 197 0.0 89 267 0.0 53 158 6.0 47 140 2.7 38 112 0.0 41 12.0 0.0 45 134 0.0 67 204 0.0 89 267 0.0 53 158 6.1 46 140 2.8 38 115 0.0 41 127 0.0 45 140 0.0 89 267 0.0 53 150 6.3 45 140 0.0 41 127 0.0 45 140 0.0 89 267 0.0 53 160 6.3 45 140 0.0 41 130 0.0 45 143 0.0 67 204 0.0 89 267 0.0 53 169 6.		1339	5.8	49	140	2.6	38	108	0.0	41	119	0.0	45	129	0.0	67	194	0.0	89	258	0.0	53	153	99	19
6.0 47 140 2.7 38 112 0.0 41 123 0.0 45 134 0.0 67 200 0.0 89 267 0.0 53 158 6.1 46 140 2.8 48 113 0.0 41 127 0.0 45 140 0.0 67 204 0.0 89 272 0.0 53 165 6.3 46 140 2.8 38 117 0.0 41 129 0.0 45 140 0.0 67 210 0.0 89 280 0.0 53 165 6.5 44 140 2.8 38 117 0.0 45 145 0.0 67 210 0.0 89 280 0.0 53 168 6.5 44 140 0.0 45 145 0.0 67 214 0.0 89 280 0.0 53 </td <td></td> <td>1302</td> <td>5.9</td> <td>48</td> <td>140</td> <td>2.7</td> <td>38</td> <td>110</td> <td>0.0</td> <td>41</td> <td>121</td> <td>0.0</td> <td>45</td> <td>132</td> <td>0.0</td> <td>29</td> <td>197</td> <td>0.0</td> <td>83</td> <td>263</td> <td>0.0</td> <td>53</td> <td>155 (</td> <td>,</td> <td>16</td>		1302	5.9	48	140	2.7	38	110	0.0	41	121	0.0	45	132	0.0	29	197	0.0	83	263	0.0	53	155 (,	16
6.7 4.6 1.2 3.0 4.7 1.2 0.0 45 1.2 0.0 45 1.2 0.0 45 1.2 0.0 45 1.0 0.0 67 2.0 0.0 89 2.6 0.0 53 163 6.3 4.5 140 2.8 3.8 117 0.0 41 131 0.0 45 140 0.0 69 280 0.0 53 168 6.5 4.4 140 2.8 38 121 0.0 41 131 0.0 45 145 0.0 67 210 0.0 89 280 0.0 53 174 6.6 4.2 140 2.9 38 126 0.0 41 130 0.0 45 145 0.0 67 220 0.0 89 280 0.0 53 174 6.6 4.2 140 0.0 67 270 0.0 <		1266	6.0	47	140	2.7	38	112	0.0	4 1	123	0.0	45	134	0.0	67	200	0.0	68 8	267	0.0	53	851	99	9 4
6.3 45 140 2.8 38 117 0.0 41 129 0.0 45 140 0.0 67 210 0.0 89 280 0.0 53 168 6.4 44 140 2.8 38 119 0.0 41 131 0.0 45 145 0.0 67 214 0.0 89 289 0.0 53 174 6.6 43 140 2.9 38 125 0.0 41 133 0.0 67 274 0.0 89 289 0.0 53 174 6.6 42 140 2.0 45 145 0.0 67 224 0.0 89 289 0.0 53 174 6.8 42 140 2.0 41 137 0.0 45 152 0.0 67 224 0.0 89 380 0.0 53 181 6.9 <td></td> <td>1199</td> <td>6.2</td> <td>46</td> <td>140</td> <td>2.8</td> <td>38</td> <td>115</td> <td>0.0</td> <td>41</td> <td>127</td> <td>0.0</td> <td>45</td> <td>138</td> <td>0.0</td> <td>67</td> <td>207</td> <td>0.0</td> <td>68</td> <td>276</td> <td>0.0</td> <td>53</td> <td>63</td> <td>99</td> <td>2 2</td>		1199	6.2	46	140	2.8	38	115	0.0	41	127	0.0	45	138	0.0	67	207	0.0	68	276	0.0	53	63	99	2 2
6.4 4.4 140 2.8 38 119 0.0 41 131 0.0 45 143 0.0 67 214 0.0 89 285 0.0 53 148 6.6 4.4 140 2.8 38 123 0.0 41 135 0.0 45 145 0.0 67 27 0.0 89 289 0.0 53 174 6.6 4.2 140 2.9 38 125 0.0 41 135 0.0 45 140 0.0 67 224 0.0 89 298 0.0 53 174 6.8 4.2 140 2.0 41 135 0.0 45 150 0.0 67 224 0.0 89 303 0.0 53 181 6.9 4.1 140 0.0 41 143 0.0 45 152 0.0 89 308 0.0		1166	6.3	45	140	2.8	38	117	0.0	41	129	0.0	45	140	0.0	67	210	0.0	83	280	0.0	53	991	99	1
6.5 44 140 2.8 38 121 0.0 41 153 0.0 45 149 0.0 67 217 0.0 89 294 0.0 53 174 6.6 4.2 4.2 4.0 2.9 38 125 0.0 41 135 0.0 45 149 0.0 67 224 0.0 89 294 0.0 53 174 6.8 4.2 140 2.9 38 125 0.0 45 169 0.0 67 224 0.0 89 294 0.0 53 174 6.9 41 140 0.0 45 150 0.0 67 224 0.0 89 303 0.0 53 181 7.0 40 140 3.0 41 143 0.0 45 156 0.0 67 230 0.0 89 312 0.0 53 181		1135	6.4	44	140	2.8	38	119	0.0	14	131	0.0	45	143	0.0	67	214	0.0	68	285	0.0	53	168	90	
6.7 4.2 14.0 2.0 41.1 15.0 45.1 49.1 15.0 47.1 47.2 47.1 47.2 47.2 47.1 89.2 37.2 67.0 89.3 37.2 67.0 53.1 17.2 17.2 47.1 47.2 47		1073	0.0	4 4 6	140	ν.ν	0 00	12.3		4 4	135	0.0	45	147	2 0	67	220	0.0	0 0	203	0.0	53	1/1	0 19	$\tilde{\alpha}$
6.8 4.2 140 2.9 38 126 0.0 41 159 0.0 45 152 0.0 67 227 0.0 89 303 0.0 53 179 6.9 41 140 3.0 41 141 2.0 41 141 0.0 45 154 0.0 67 230 0.0 89 307 0.0 53 181 7.0 40 140 3.0 41 143 2.0 45 156 0.0 67 234 0.0 89 312 0.0 53 184 7.1 40 140 3.0 45 156 0.0 67 244 0.0 89 326 0.0 53 189 7.2 40 140 3.0 45 165 0.0 67 244 0.0 89 326 0.0 53 192 7.4 40 146 3.2 </td <td></td> <td>1044</td> <td>6.7</td> <td>42</td> <td>140</td> <td>2.9</td> <td>38</td> <td>125</td> <td>0.0</td> <td>41</td> <td>137</td> <td>0.0</td> <td>45</td> <td>149</td> <td>0.0</td> <td>67</td> <td>224</td> <td>0.0</td> <td>68</td> <td>298</td> <td>0.0</td> <td>53</td> <td>176</td> <td>3 9</td> <td>2 ∞</td>		1044	6.7	42	140	2.9	38	125	0.0	41	137	0.0	45	149	0.0	67	224	0.0	68	298	0.0	53	176	3 9	2 ∞
6.9 41 140 3.0 41 141 0.0 45 154 0.0 67 230 0.0 89 307 0.0 53 181 7.0 40 140 3.0 41 143 0.0 45 156 0.0 67 234 0.0 89 312 0.0 53 184 7.1 40 140 3.1 42 148 2.1 41 145 0.0 45 160 67 234 0.0 89 312 0.0 53 184 7.2 40 140 3.0 45 160 60 67 244 0.0 89 326 0.0 53 189 7.4 40 146 3.2 41 143 0.0 45 160 67 244 0.0 89 326 0.0 53 189 7.4 40 146 3.2 41 151		1015	8.9	42	140	2.9	38	126	0.0	41	139	0.0	45	152	0.0	67	227	0.0	89	303	0.0	53	179	99	1 20
7.0 40 140 3.0 41 143 0.0 45 156 0.0 67 234 0.0 89 312 0.0 53 184 7.1 40 140 3.0 41 145 0.0 45 158 0.0 67 240 0.0 89 316 0.0 53 189 7.2 40 140 3.1 42 148 2.1 41 147 0.0 45 163 0.0 67 240 0.0 89 326 0.0 53 189 7.3 40 146 3.2 42 150 2.1 41 147 0.0 45 163 0.0 67 240 0.0 89 329 0.0 53 192 7.4 40 146 3.2 41 151 0.0 45 162 0.0 89 334 0.0 53 192 7.5 <td></td> <td>986</td> <td></td> <td>41</td> <td>140</td> <td>3.0</td> <td>41</td> <td>141</td> <td>2.0</td> <td>41</td> <td>141</td> <td>0.0</td> <td>45</td> <td>154</td> <td>0.0</td> <td>67</td> <td>230</td> <td>0.0</td> <td>89</td> <td>307</td> <td>0.0</td> <td>53</td> <td>181</td> <td>99</td> <td>150</td>		986		41	140	3.0	41	141	2.0	41	141	0.0	45	154	0.0	67	230	0.0	89	307	0.0	53	181	99	150
7.1 40 140 3.1 42 142 3.1 42 143 3.1 42 148 2.1 41 143 0.0 45 150 2.7 60 89 320 0.0 53 189 7.2 40 143 3.1 42 150 2.1 41 149 0.0 45 163 0.0 67 244 0.0 89 329 0.0 53 192 7.4 40 146 3.2 42 150 2.2 41 151 0.0 45 165 0.0 67 244 0.0 89 326 0.0 53 192 7.5 40 146 3.2 41 151 0.0 45 165 0.0 67 247 0.0 89 334 0.0 53 192 7.5 40 156 2.2 41 153 0.0 45 169 0.0 </td <td></td> <td>957</td> <td>7.0</td> <td>40</td> <td>140</td> <td>3.0</td> <td>4 5</td> <td>143</td> <td>2.0</td> <td>4</td> <td>143</td> <td>0.0</td> <td>45</td> <td>156</td> <td>0.0</td> <td>67</td> <td>234</td> <td>0 0</td> <td>68</td> <td>312</td> <td>0.0</td> <td>53</td> <td>84</td> <td>9 9</td> <td><u>စ</u>ါင္</td>		957	7.0	40	140	3.0	4 5	143	2.0	4	143	0.0	45	156	0.0	67	234	0 0	68	312	0.0	53	84	9 9	<u>စ</u> ါင္
7.3 40 143 3.1 42 150 2.1 41 149 0.0 45 163 0.0 67 244 0.0 89 325 0.0 53 192 7.4 40 146 3.2 42 153 2.2 41 151 0.0 45 165 0.0 67 247 0.0 89 329 0.0 53 192 7.5 40 148 3.2 42 153 2.2 41 153 0.0 67 250 0.0 89 334 0.0 53 192 7.6 40 150 3.3 42 157 2.0 45 169 0.0 67 254 0.0 89 334 0.0 53 200 7.8 40 155 3.2 41 157 0.0 45 174 0.0 67 254 0.0 89 343 0.0 53 </td <td></td> <td>929</td> <td>7.7</td> <td>4</td> <td>141</td> <td></td> <td>42</td> <td>2 4 6</td> <td>2.1</td> <td>1 4</td> <td>14.7</td> <td>0.0</td> <td>5.4</td> <td>001</td> <td>0.0</td> <td>67</td> <td>240</td> <td>0.0</td> <td>0 00</td> <td>320</td> <td>0.0</td> <td>53</td> <td>0 0</td> <td>2 9</td> <td>2 2</td>		929	7.7	4	141		42	2 4 6	2.1	1 4	14.7	0.0	5.4	001	0.0	67	240	0.0	0 00	320	0.0	53	0 0	2 9	2 2
7.4 40 146 3.2 42 151 0.0 45 165 0.0 67 247 0.0 89 329 0.0 53 195 7.5 40 148 3.2 42 153 0.0 45 167 0.0 67 250 0.0 89 334 0.0 53 197 7.6 40 150 3.3 42 157 2.3 41 155 0.0 45 169 0.0 67 254 0.0 89 343 0.0 53 200 7.8 40 150 2.3 41 157 0.0 45 174 0.0 67 254 0.0 89 343 0.0 53 205 7.8 40 156 3.4 159 0.0 45 174 0.0 67 264 0.0 89 347 0.0 53 205 8.0 41 <td></td> <td>874</td> <td>7.3</td> <td>40</td> <td>143</td> <td>3.1</td> <td>42</td> <td>150</td> <td>2.1</td> <td>41</td> <td>149</td> <td>0.0</td> <td>45</td> <td>163</td> <td>0.0</td> <td>67</td> <td>244</td> <td>0.0</td> <td>89</td> <td>325</td> <td>0.0</td> <td>53</td> <td>192</td> <td>99</td> <td>12</td>		874	7.3	40	143	3.1	42	150	2.1	41	149	0.0	45	163	0.0	67	244	0.0	89	325	0.0	53	192	99	12
7.5 40 148 3.2 42 153 0.0 45 167 0.0 67 250 0.0 89 334 0.0 53 197 7.6 40 150 3.3 42 157 2.3 41 155 0.0 45 169 0.0 67 254 0.0 89 338 0.0 53 200 7.7 40 150 3.3 42 169 2.3 41 157 0.0 67 257 0.0 89 343 0.0 53 202 7.8 40 155 3.4 159 0.0 45 174 0.0 67 264 0.0 89 347 0.0 53 205 7.9 40 158 3.5 42 165 2.5 41 161 0.0 45 176 0.0 67 264 0.0 89 357 0.0 53 208<		845	7.4	40	146	3.2	42	153	2.2	41	151	0.0	45	165	0.0	67	247	0.0	83	329	0.0	53	195	99	읾
7.7 40 152 3.3 42 159 2.3 41 157 0.0 45 172 0.0 67 254 0.0 89 343 0.0 53 202		707	ر: ۲	4	248	5.2	4.7	155	7.7	4 /	155	2 0	τ γ	16/	2 0	79	250	2 0	2000	554	0.00	55 A	/6	0 9	3 5
7.8 40 155 3.4 42 162 2.4 41 159 0.0 45 174 0.0 67 260 0.0 89 347 0.0 53 205 205 7.9 40 158 3.5 42 165 2.5 41 161 0.0 45 176 0.0 67 264 0.0 89 352 0.0 53 208 8.0 41 161 3.7 43 169 2.7 41 163 0.0 45 178 0.0 67 267 0.0 95 377 2.1 53 210		756	7.7	4	152	3.3	42	159	2.3	4	157	0.0	45	172	0.0	67	257	0.0	68	343	0.0	53	202	2 9	715
7.9 40 158 5.5 42 165 2.5 41 161 0.0 45 176 0.0 67 267 0.0 89 552 0.0 53 208 80 41 161 3.7 43 169 2.7 41 163 0.0 45 178 0.0 67 267 0.0 95 377 2.1 53 210		723	7.8	40	155	3.4	42	162	2.4	41	159	0.0	45	174	0.0	29	260	0.0	83	347	0.0	53	-	99	21
		683	ς. Σ. α	4	161	5.5	47	169	2.5	4 4	163	0.0	45	178	0 0	67	264	0.0	99 7	377	0.0	+	+	0 9	2 2
		1	5		2						2								:			+	-		T
		CR, LS &		.UES	N FE	T. L!	STED	RADIUS	SIS	밀	MOMIN.	ALLO	WABLE	RADIL	JS FO	R TH	COR	RESPO	NUING	E. CR	S	× ⊆	VALLE	s.	
CR, IS & w VALUES IN FEET. LISTED RADIUS IS THE MINIMUM ALLOWABLE RADIUS FOR THE CORRESPONDING E, CR, LS, AND w VALUES.																			į	ĵ		<u>:</u>	,		

TRANSITION CURVES - RURAL 45 MPH DESIGN SPEED

.01	RAMPS	FT	LS	09 5	63	69	72	78	8	84 7	06	93	66	105	108	111	117	117	120	123	126	132	135	141	144	150	153	159	16.5	168	171	177	180	186	193	195	198	204	207	210	216	219	225	228	234	237	240
		2	8 6	09	09	09	09	09	09	09	09	909	09	09	09	09	09	09	09	09	90	09	09	09	09	09	09	09	09	09	90	09	90	09	90	09	09	09	09	9	09	09	09	09	09	09	09
MAX.	INTERCHANGE	H	LS	57	59 62	65	68	73	9/	79	85	88	93	96	102	107	110	110	113	116	118	124	127	132	135	141	144	149	152	158	160	166	169	175	177	183	186	191	194	797	203	205	211	214	219	222	225
X X X X X X X X X X	INTE	16	S.	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57
S	F	12.	≥ 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0:0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
S	WIDTH=72	3 @ 1	LS	96	101	111	116	125	130	135	144	154	159	164	173	178	188	188	192	197	202	212	216	226	231	240	245	255	260	269	274	284	288	298	303	312	317	322	332	336	346	351	360	365	375	380	384
NSING	MD		8	96	96 96	96	96	96	96	96	96	98	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96	96
(AR)	F E	12'	> 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	C
(RURAL)	WIDTH=48 F	2 @ 1	LS	72	76	83	87	96	86	101	108	112	119	123	130	134	141	141	4 4 4 4 4 4	148	15.5	159	162	170	173	180	184	191	195	202	206	213	216	224	227	234	238	242	249	252	260	263	270	274	281	285	288
H M M	-		R c	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72 / 22	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72	72/	72	72	72	72	72	72	72	72	72
	FT AT		≥ 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	C
50	WIDTH=24	1 @ 12	LS	48	53	26	28	63	65	68	72	75	80	82	87	89	94	94	96	66	101	106	108	113	116	120	123	128	130	135	137	142	144	149	152	156	159	164	166	168	173	176	180	183	3 28	190	102
	WID		8	48	4 8 4 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	48	8 4	0 4 6	48	4 0 0	4 8	8 4 8	48	8 4 8	48	8 4 8	84	48	φ φ φ	48	4 8 X	48	4 8	4 8 8	φ 4 α	4 8	φ 4 α α	48	8 4	48	80 4	48	8 4	48	8 4	8	48	φ 4 8 8	4 4	φ 4	4 8	_		φ 4	48	\perp	Α
SPEED	WIDTH=22 FT WIDTH=24 FT		> 0	0.0	0.0	0:0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0:0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0:0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	C
	WIDTH=22	10 11	LS (0 4	47	51	53	28	09	62	99	69	73	75	80	82	98	86	88	91	93	97	99	104	106 a 01	9 2	113	112	119	124	126 128	130	132	137	139	143	146	150	152	154	159	161	165	168	172	174	176
IVI		5	8	+	4 4 4 4	4 4	4 4	_	44	4 4	+	4 4 4 4	44	4 4 4	4 4	4 4 4	44	44	4 4	44	4 4 4	4 4	4 4	4 4 4	4 4	† † † †	4 4 4 4	4 4	4 4 4	4 4	4 4 4	4 4	4 4 4	4 4	4 4 4	44	44	4 4 4	4 ;	4 4	† † 4	44	4 4 4	4 4	44	44	44
			> 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.0	2.1	2.1	2.2	2.3	c v
FOR A	FT WIDTH=20 F	1 0 10	LS	9 4	42	46	8 4	52	54	29	09	62	99	89 0	72	74	78	78	8 8	82	84	88	8	94	96	100	102	106	108	112	114	118	120	124	126	130	132	136	138	140	160	161	166	168	174	177	α.
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TOR!	18 F.			0 0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.2	2.2	2.2	2.2	2.3	2.3	2.3	2.4	2.4	2.4	2.5	2.5	2.5	2, 0	2.6	2.6	2.7	2.7	2.8	2.8	2, 2	2.9	2.9	3.0	ω, r	3.1	3.1		W. 1	_
FACTORS	WIDTH=	1 @ 9		36	38	42	44	t 4	49	51	54	56	8 9	62	65	67	71	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	160	-	160	161	166	169	177
	≥		0	36	36	36	36	36	36	36	36	36	36	36	36	36	36	83	8 8	79	77	73	72	0/	67	64	63	61	09	58	57	55	54	52	12 5	202	49	4 4 α α	47	46	45	44	444	43	43		4 3
DESIGN	> z) E(%)	2.0	2.1	2.3	2.4	2.6	2.7	2.8	3.0	3.7	3.3	3.5	3.6	3.7	3.9	3.9	0.4 0.0	4.1	4.2	4.4	4.5	4.7	4 4 8 0	5.0	5.2	5.3	5.4	5.6	5.7		6.0	6.2	6.3	6.5	6.6	\. 6.8	6.9	7.1	7.2	7.3	7.5	7.6	7.8	7.9	0.00
	DESIGN VFI OCITY	-20	RADIUS(FT)	6013	5703	5162	4926	4700	4320	4146	3834	3692	3434	3316	3099	2999	2866	2865	2728	2646	2568	2422	2353	2224	2163	2047	1992	1888	1838	1743	1698	1610	1568	1487	1448	1372	1336	1265	1230	1196	1128	1094	1024	988	908	860	760

TRANSITION CURVES - RURAL 50 MPH DESIGN SPEED VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

MPS	H	- 0	3 0	64	/9	73	2/6/	83	98	93	95	86	105	108	111	114	120	124	130	133	130	143	143	146	149	155	158	162	168	171	177	180	197	193	199	203	206	212	215	222	228	231	234	240	247	250	ЗТС
SE RA	c	© 6	50	64	64	64	64	64	64	40	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	t 5
CHAN	WIDTH	- 0	3 0	09	63	69	75	78	80	20 80	89	92	26 8	5	104	107	113	116	122	125	131	134	134	137	140	146	149	15.5	158	160	166	169	178	18	187	190	193	199	202	208	211	217	220	226	232	235	7007
INTERCHANGE RAMPS		9 9	50	09	09	09	09	09	09	2 6	09	09	09	3 9	09	09	09	09	09	09	2 0	09	09	09	09	3 6	09	09	09	09	3 9	09	09 09	09	2 09	09	09	09	09	09	09	09	09	9 9	09	09	20
FT		3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2 0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
WIDTH=72	0		3 0	103	108	118	128	133	138	140	154	159	164	174	179	184	195	200	205	215	220	230	230	235	240	251	256	261 266	271	276	286	292	302	312	322	327	332	343	353	358	368	373	378 383	389	399	404	1 0 0
MIDT	1	ر ا	50	103	103	103	103	103	103	103	103	103	103	103	103	10.3	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	103	5
F	÷	3	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	- ?
WIDTH=48	LANE WIDTH)	2 0	3 0	77	81	68	96	100	104	120	115	119	123	131	135	138	146	150	158	161	169	173	173	177	180	188	192	196	203	207	215	219	226	234	242	246	249	257	261	269	272	280	284	292	299	303	, ,
WIDT	LANE	7 0	50	77	//	77	77	77	77	//	77	77	77	77	77	77	77	77	//	77	//	77	77	77	77	//	77	77	77	77	77	77	77	12	//	77	77	77	77	77	//	77	77	77	77	77	-
	S AT	3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
	LANES	7 0	3 0	52	57	59	64	67	69	75	77	80	828	87	06	92	86	00	105	108	113	115	115	138	120	126	128	131	136	138	43	946	151	156	161	164	166	172	174	179	84	87	192	195	200	202	3
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			× 0.0		0.0		0.0				0.0	0.0	0 0	0.0	0.0	0.0	0.0	- 1	0.0	0.0	0 0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	2.1	2.5	?
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VIRGINIA DEPARTMENT OF TRANSPORTATION

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 | |
| FT | | | 0:0 | 0.0 | 0.0 | 0.0
 | o c | 0.0 | 0.0 | 0.0 | 0.0
 | 0.0 | 0.0 | 0 0 | 0.0 | 0.0
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 | |
| TH=22 | FN 5 | ST | 0 | 52 | 57 | 59
 | 62 | 99 | 69 | 74 | 9/
 | 81 | 98 | 98 | 93 | 96
 | 101 | 103 | 108 | 110 | 115 | 118 | 123
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128 | 128 | 130 | 132 | 137
 | 140 | 145 | 150 | 154 | 157 | 162 | 164 | 169 | 172 | 176 | 181 | 184 | 189
 | |
| MID | OIIV AL | SR | 0 | 49 | y 4
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| | | - 1 | 0.0 | 0.0 | 0.0 | 0.0
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 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0
 | |
| IH=20 | N N N | LS | 0 | 47 | 52 | 54
 | 20 00 | 99 | 63 | 67 | 69
 | 74 | 78 | 80 | 85 | 87
 | 92 | 94 | 88 | 100 | 105 | 107 | 112
 | 116 | 116 | 118 | 120 | 125
 | 127 | 132 | 136 | 140 | 14.3 | 14.7 | 149 | 154 | 158 | 160 | 165 | 167 | 172
 | |
| QIM : | N. S. | - 1 | 0 | 45 | 45 | 45
 | 45 | 45 | 45 | 45 | 45
 | 45 | 45 | 45 | 45 | 45
 | 45 | 45 | 45 | 45 | 45 | 45 | 45
 | 45 | 45 | 45 | 45 | 45
 | 45 | 45 | 45 | 45 | 45 | 45 | 45 | 45 | 45 | 45 | 45 | 45 | 45
 | |
| FT | DE SI | > | 0.0 | 0.0 | 0.0 | 0.0
 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0
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 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0
 | 0.0 | 2.3 | 2.4 | 2.4 | 2.4
 | 2.5 | 2.5 | 2.5 | 2.6 | 2.6 | 2.6 | 2.6 | 2.7 | 2.7 | 2.8 | 7.8 | 2.9 | 2.9
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| MID | - | CR | 0 4 | 04 0 | 04 | 04 9
 | 404 | 2 4 | 40 | 04 40 | 40
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2.0 | 2.1 | 2.3 | 4.7
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م | 4.7 | 8.4 | 2.0
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 | |
| | WIDTH- 18 FT WIDTH-20 FT WIDTH-22 FT WIDTH-24 FT WIDTH-48 FT WIDTH-72 FT INTERCH | 18 FT WIDTH-20 FT WIDTH-22 FT WIDTH-24 FT WIDTH-48 FT WIDTH-72 FT INTERCHANGE DESIGN SOFTWARE EQUIVALENTS (NUMBER OF LANES AT LANE WIDTH) WIDTH 10 10 11 10 11 10 17 1 | SITY MIDTH= 18 FT MIDTH=22 FT MIDTH=22 FT MIDTH=48 FT MIDTH=48 FT MIDTH=72 FT | VIDTH= 18 FT VIDTH=20 FT VIDTH=22 FT VIDTH=24 FT VIDTH=48 FT VIDTH=48 FT VIDTH=47 FT VIDTH=72 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=72 FT MIDTH=70 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=46 FT MIDTH=47 FT MIDTH=72 FT MIDTH=70 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=72 FT MIDTH=72 FT MIDTH=70 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=72 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=72 FT MIDTH=70 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=72 FT MIDTH=70 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=40 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=46 FT | MIDTH= 18 FT MIDTH=20 FT MIDTH=22 FT MIDTH=24 FT MIDTH=48 FT MIDTH=48 FT MIDTH=72 FT MIDTH=27 FT MIDTH=40 FT | MICH 18 FT MIDTH-20 FT MIDTH-22 FT MIDTH-24 FT MIDTH-27 FT | MIDTH- 18 FT MIDTH-20 FT MIDTH-22 FT MIDTH-24 FT MIDTH-48 FT MIDTH-72 FT MIDTH-70 FT | MIDTH+ 18 FT MIDTH+20 FT MIDTH+22 FT MIDTH+42 FT MIDTH+48 FT MIDTH+74 FT MIDTH+72 FT MIDTH+72 FT MIDTH+74 FT MIDTH+72 FT MIDTH+72 FT MIDTH+74 FT MIDTHH-74 FT MIDTH+74 FT | MIDTH- 18 FT MIDTH-20 FT MIDTH-22 FT MIDTH-24 FT MIDTH-48 FT MIDTH-17 FT MIDTH-NIGER MIDTH-NIGH MIDTH-NI | MIDTH | MIDTH-18 F MIDTH-20 F MIDTH-22 F MIDTH-24 F MIDTH-448 F MIDTH-48 F MIDTH-42 F MIDTH-40 F MI | MINITAL 18 T MINITAL T MINITAL | MOTH+ | MINTH S T MINTH S T MINTH S T MINTH S T MINTH MINT | MINTH- 18 FT MINTH-20 FT MINTH-22 FT MINTH-24 FT MINTH-24 FT MINTH-27 FT MINTH-24 FT MINTH-27 FT MINTH-24 FT MINTH-27 FT MINTH-26 FT MINTH-27 FT MINTH-26 FT MINTH-27 FT MINTH-27 | MINTH- 18 T MINTH-20 T MINTH-20 T MINTH-22 T MINTH-22 T MINTH-24 T MINTH-22 T MINTH-20 T MIN | MINTH- 18 T MINTH- 20 T MINTH- 20 | MINTH- 18 FT MINTH-20 FT MINTH-22 FT MINTH-24 FT MINTH-36 FT | Fig. 2 Fig. 3 Fig. 4 F | Fig. 2 Fig. 3 F | Fig. 2 Fig. 3 F | Model Mode | Moth | Mainty- B T Mainty- D T Mainty- D T Mainty- D T Mainty- B T Mainty- D T Main | | | Column C | Moith | | | | | Month St Mon | Mathematical Mat |

TRANSITION CURVES - RURAL 60 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

	SAMPS	FT	LS C	202	5/ 2/	808	87	94	97	101	108	111	TI 8E	122	125	132	135	142	146	15.3	156	160	167	170	174	180	187	191	194	201	201	205	212	215	222	229	232	239	243	246	253	260	267	270	274	TC-5.01
PH (RURAL) USING E= 8	GE RA	∞	ي ج د	2 2 2	2/2/	8 8	70	2/2	70	0/2	2 2	22	5 5	70	0/2	70	70	70	2/2	2/2	70	2/2	2 2	2/2	0/	70	2/2	70	5 5	07	2/2	0/2	70	9 2	70	2/2	70	70	22	0 02	70	70	5 5	70	5 5	JES.
	CHANGE	FT	LS C	99	72	79	82	89	92	95	102	105	112	115	178	125	128	134	138	144	147	151	157	160	164	170	177	180	183	061	190	193	200	203	209	216	219	226	229	232	239	245	252	255	258	w VALUES
	INTER	16	8 0	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	99	AND »
	FT		» C	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	LS,
	WIDTH=72	3 @ 12	LS	112	123	134	140	151	157	162 168	174	179	190	196	207	213	218	229	235	246	252	257	268	274	280	291	302	307	313	324	324	330	341	352	358	369	374	386	391	397 402	408	419	425	436	441	; E, CR,
	MID	(4)	S, C	112	112	112	112	112	112	12	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	112	12	112	112	ONDING
	F E	1 .	> C	0.0	0.0	0.0	0.0	0.0	0.0	0 0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	CORRESPONDING
	WIDTH=48 F ANE WIDTH)		S C	84	93	101	105	114	118	122	130	134	143	147	151	160	164	172	176	185	189	193	201	206	210	218	227	231	235	243	243	247	256	264	268	277	281	289	294	302	306	314	323	327	331	
	WIDT		S C	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	48	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84	84 84	84	84	FOR THE
	FT S AT	,	> C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
	WIDTH=24		LS C	26	62	65	70	2/9/	79	8 8	87	06	95	98	101	107	109	115	118	12.3	126	129	134	137	143	146	151	154	157	162	162	165 168	171	174	179	185	187	193	196	201	204	210	215	218	221	: RADIUS
9	WIDT!		S C	56	56	26	56	26	56	56	56	56	56	56	56	56	56	56	56	56	56	56	26	56	56	56	26	56	56	56	56	56	56	26	56	56	56	56	56	56	56	56	20 20	56	56	ALLOWABLE
D 0F	FT W		» O	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	ALLO
SPEE			LS C	52	57	59	64	/0/	72	75	80	82	87	90	95	98	100	105	108	113	116	118	123	126	128	134	139	141	144	149	149	151	157	159	164	169	172	177	180	182	19.0	192	197	200	203	MINIMUM
	WIDTH=22 EQUIVALENTS		S C	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	52	THE MIT
SIGN	니		* C	0.0	0.0	0.0	0.0			0 0	0.0	0.0	0.0	0.0	0.0		0.0			0.0		- 1	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 0	0.0	0.0	<u>N</u>
FACTORS FOR	WIDTH=20 F	Ø 10	LS	74	52	56	59	63	99	89	73	75	> 08	82	87	89	94	96	98	103	105	107	112	114	117	121	126	128	133	135	135	138	142	145	149	154	156	161	163	168	170	175	180	182	184	RADIUS
	LOIW NS		S C	7 4 7	47	47	47	47	47	47	47	47	4/47	47	4 4 7	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	4/47	47	47	47	47	47	4 / 4	47	47	47	47	47	LISTED F
	18 FT V DESIGN	2	> 0	0.0	0.0	0.0	0.0			0.0		0.0	0.0	1 - 1	0.0		0.0			0.0			0.0		0.0	0.0	0.0	0.0	0.0	0.0	2.4	2.5	2.5	2.5	2.5	2.6	2.6	2.6	2.7	2.7	2.7	2.8	8.70	2.9	2.9	
	WIDTH= 18	-6 @	r C	42	47	51	53	57	59	61	65	67	72	74	76	80	82	86	88	93	95	97	101	103	105	109	114	116	118	122	200	200	200	200	200	200	200	200	200	200	200	200	200	200	200	IN FEET.
	MID		S, C	24 5	42	42	42	42	42	42	42	42	42	42	42	42	42	42	45	42	42	42	42	42	42	42	42	42	42	42	69	68	99	64	63	61	09	58	28	57	55	54	2 2	52	50	VALUES I
		•	E(;;)	2.0	2.2	2.5	2.5			2.9		3.2	5.5	3.5	3.7	3.8	3.9	4.1	4.2	J. 4	4.5	4.6	4.8	6.4	5.0	5.2	5.4	5.5	5.6	8 4	5 8	5.9	6.1	6.3	6.4	6.6	6.7	6.9	7.0	7.2	7.3	7.5	7.7	7.8	7.9	w VAL
	DESIGN VELOCITY	-65	14000	9566	8643	8242 7873	7534	6931	6662	6411	5957	5751	5557	5203	5040	4740	4601	4344	4224	4000	3896	3795	3607	3518	3433	3272	3122	3051	2982	2866	2852	2789	2670	2613	2504	2398	2346	2242	2191	2139	2034	1924	180.5	1737	1656	~
Si	PECIFIC REFERE	ATION		1 1	1 1								T				TIG									SF				?А	L						<u> </u>	1								

VIRGINIA DEPARTMENT OF TRANSPORTATION

TRANSITION CURVES - RURAL 70 MPH DESIGN SPEED

VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE